## APPENDIX FOUR STORMWATER DISCHARGE REPORT + PLANS



# SHELLY BAY STORMWATER DESIGN

Stormwater Discharge Report

#### **DOCUMENT CONTROL RECORD**

**CLIENT** Shelly Bay Taikuru Limited

**PROJECT** SHELLY BAY TAIKURU

**PROJECT NO.** 1098-01

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#### 1.0 INTRODUCTION

This Stormwater Design report has been prepared in support of an application to Greater Wellington Regional Council ("the Regional Council"), for Consent to discharge operational stormwater to land where it may enter water, from a new urban development associated with earthworks exceeding 3,000m<sup>2</sup>. The consent also seeks approval for the stormwater infrastructure including raingardens, tree pits, pipework, and inlets.

The stormwater design detailed in this report has been prepared based on the Masterplan development concept and sufficient information has been included to assess the likely stormwater runoff from the site and to make provision for the infrastructure required to properly manage it.

Water Sensitive Urban Design ("WSUD") measures such as rain gardens and tree pits have been incorporated into the stormwater design for water quality purposes.

The site extends over 12.4 Ha, and its legal description is Lots 1 – 8 DP 515825, Lot 100 DP 515825, Section 3 – 6 SO 339948, Section 10 SO 339948, Section 100 SO 528811, Lot 906 DP 548924, Lots 13 – 24 DP 548924 and Section 1 SO 419545.

The current Stormwater network is aged and of inadequate capacity for its current role, and insufficient in scope to cope with the demands of more intensive development such as that proposed. Currently no treatment is provided and the outfalls do not include adequate protection against erosion.

#### 2.0 PRELIMINARY STORMWATER DESIGN

A stormwater design concept was prepared by Envelope Engineering Limited ("Envelope") and consented as part of the Masterplan Resource Consent for the project. Wellington Water Limited reviewed the design and provided conditions and advice notes that were included in the granted consent.

The design includes a gravity system, incorporating drainage for the wider upland catchment as well as for the development site itself. The stormwater design including the extent of services required and likely runoff rates is based on the Masterplan plans guidelines and the Masterplan resource consent conditions.

As part of the design, the inclusion of WSUD measures has been provided for and these have been included as part of the landscaping design and provisions.

These have been specifically detailed as part of the Public Domain (road and esplanade) area of the project and will be required to be included in the hard stand areas of the development lots (by condition).

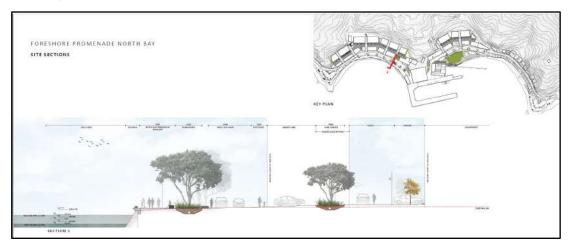


Fig 2.1: Development Concept



#### 2.1 OVERALL CONCEPT DETAILS

The overall stormwater concept is summarised in the following points:

- Any areas of existing road (i.e. Shelly Bay Rd between Miramar cutting and main Shelly Bay development area) that are upgraded and/or provided with a shared pathway will remain as is. That means few catchpits or treatment, and stormwater generally sheds off the road through vegetation, towards the Coastal Management Area (CMA) (including outflows from the catchpits);
- Road runoff within the Shelly Bay Development will drain away from the CMA to raingardens as illustrated on the stormwater plans.
- Treatment is proposed for most trafficable areas, and as much of the non-trafficable areas as is
  practicable. Given the nature of the non-trafficable areas (being predominantly pedestrian areas),
  contaminant generation levels will be low and are not considered to generate environmental
  effects that would necessitate stormwater treatment mitigation.
- No roof materials will be zinc or copper in accordance with the conditions of the Masterplan resource consent conditions and therefore it is not necessary to treat runoff from the buildings;
- The private areas of roadway and hard stand (i.e. those in and around the buildings) will have coarse sediment traps installed (for example 'Litta Traps' or 'Enviropods') within catchpits and, where practicable, the majority of these areas will be treated. Treatment is not likely to be provided for small areas of the laneway because flows are not able to be directed to the roadway without amending the masterplan concept. The specific areas include short sections of laneway between the vehicle crossing and the internal parking zones which are required to be separately drained and not permitted to discharge onto the road (where they would otherwise have been collected by proposed rain garden features). The project engineers are confident that the discharge will not contain more than 15 milligrams per litre of total petroleum hydrocarbons prior to release (refer condition (ii) of Rule R48).
- The carpark areas at South and North Bay which existed but are upgraded will remain gravel and semi permeable. Refer application drawing 1098-01-GW805 that notes that 'flows from carpark will drain towards rocky coastal planting that will act as filter traps for sediment control prior to runoff entering the coastal management area'.
- Upstream surface stormwater flows will be passed through/ between the development out to the CMA in designated overland flowpaths; (generally will be collected into the proposed stormwater system and discharged) and,
- No stormwater detention is proposed due to proximity to the discharge point and that no watercourses will be affected.

#### 2.2 STORMWATER DISCHARGE

Stormwater design has been designed to the Wellington Water "Regional Standard for Water Services 2019", with the general design to a 10% AEP rainfall level including 20% increase for climate change. This has resulted in a design rainfall intensity (for Tc=10 minutes) of 84.5 mm/hr, based on HIRDS v4.0, RCP6.0.

There are significant upland catchments in the escarpments above the sites. These in turn however do not result in permanent water flows, but in one gully an intermittent water course has been identified (catchment E2). Refer map of upland catchments in Fig 2.1.1 below.



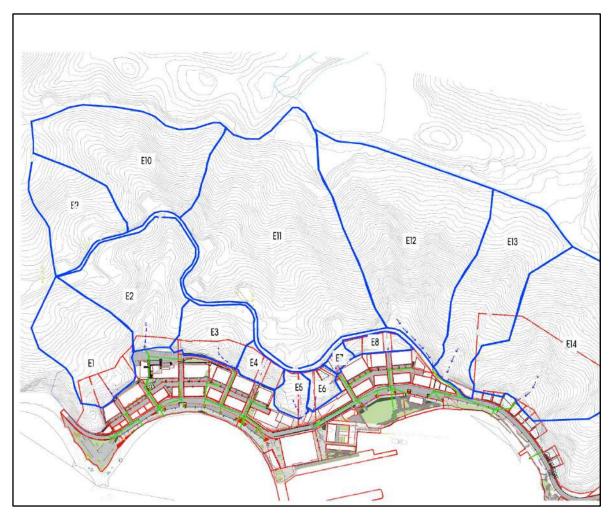


Fig 2.1.1: Upland catchments

Anticipated runoff for the various catchments has been assessed as follows:

- i. For the upland catchments a HEC-HMS analysis has been undertaken utilising the recommendations of "Reference Guide for Design Storm Hydrology" prepared for Wellington Water by Cardno; 2019. This has included a CN number of 54 applying to the total area, as recommended in the report, and an Initial Abstraction of 21.6mm based on 10% St
- ii. For the development lots and public realm catchments a Rational Analysis utilising a C value of 0.95 for impermeable areas.

All contributing catchments are currently managed with an outdated stormwater system, that does not fit with the form of the proposed development.

A new network feeding to six outfalls has been proposed, with pipe sizes ranging from 300mm to 675mm diameter as indicated on the plans appended. As the designs for the development lots have not yet been finalised, details relating to private connections (including building and hardstand drainage connections) may vary.

The outfalls discharge at rates ranging from  $52.17 \, \text{L/s}$  to  $339.25 \, \text{L/s}$  (refer appendix) for the  $10\% \, \text{AEP}$  Event.



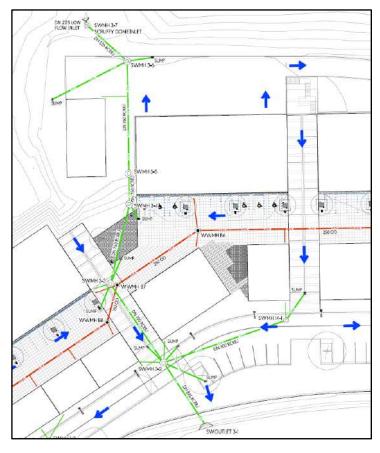


Fig 2.1.2: Proposed Stormwater Drainage Detail

Due to the proximity of the site to the coast, and in accordance with pre-application discussions, hydraulic neutrality has not been adopted.

#### 2.3 EPHEMERAL STREAM OVERFLOW STRUCTURE

The outlet to the ephemeral stream which discharges from catchment E2, incorporates a pipe and headwall with capacity for a 10% AEP storm event (with climate change). Behind this a riser weir with scruffy dome will have a capacity for 1% AEP events to avoid erosion of the stream bank.

The 1% AEP event increases runoff from 217 L/s to 510 L/s. This has resulted in the stormwater line to the outfall increasing in capacity to manage this additional flow.

The scruffy dome intake will be able to cater for this flow with a water level at 50mm above the top of the manhole.

This form of intake minimises the extent of encroachment into the stream bed to no more than 3-5m.

#### 2.4 STORMWATER OUTFALLS

New stormwater lines are to be constructed with coastal outfalls, either replacing existing mains or construction of new lines to accommodate the additional runoff generated. Within the site the construction will be managed by the erosion and sediment control provisions described in the draft Earthworks and Construction Management Plan. Additional measures will be required to accommodate the specific requirements of the outfall structures.

These will be integral with the upgraded seawall structures and therefore will be included in the seawall upgrade resource consent. While these works are not covered in the GWRC discharge permit for operational stormwater, the following information has been provided for reference.



#### 2.4.1 OUTFALL CONSTRUCTION

The current intention is to construct the seawalls as flexible structures, incorporating rip rap, which adjusts to settlement; it also serves to trap sediment and reduce flow velocities.

The nature of the stormwater runoff will include a combination of upland runoff from undeveloped vegetated hills above the site, and to a lesser extent treated stormwater runoff from the development lots and the road below. As a result we can confirm that the nature of the discharge will be in accordance with the Permitted Activity rules.

Riprap aprons should be constructed, where possible, at zero percent grade for the specified length. In general, ungrouted, properly sized riprap provides better assurance of long-term performance. Filter cloth laid between the soil and riprap to minimise the likelihood of soil erosion at the interface.

Construction of the outfall protection must be done at the same time as construction of the pipe outfall itself.

Generally, it is best to construct the outfall unit from the bottom up, to prevent concentrated flows from being discharged into an unstabilised location. Where the outfall is part of a replacement system, the existing outfall may be able to be utilised during the construction phase, If construction of the outfall system is done from the top end first, the entrance to the system should be blocked off to prevent flow from travelling through the pipe until the outfall protection is completed.

It is important that a sequence of construction be established and followed, such as, for example:

- 1. The foundation area will be cleared of trees, stumps, roots, grass, loose rock, or other unsuitable material.
- 2. The cross-section will be excavated to design with over-excavated areas backfilled with moist soil compacted to the density of the surrounding material.
- 3. Abrupt deviations from the design grade or horizontal alignment will be avoided.
- 4. Filter cloth and riprap will be laid line and grade and, in the manner specified. Sections of fabric should overlap at least 300 mm and extend 300 mm beyond the rock. The filter cloth will be secured at the edges via secure pins or a key trench.
- 5. The construction operations will be carried out to minimise erosion or water contamination, with all disturbed areas vegetated or otherwise protected against soil erosion. Rip Rap to be clean metal without included silt or clay. Temporary access to the coastal area should be constructed of similar material to avoid silt contamination.
- 6. Construction will be carried out at periods of low tide with progressive stabilisation at each stage to avoid erosion.





Fig 2.3.1: Typical Coastal Outfall Under Construction.

#### 3.0 WATER SENSITIVE URBAN DESIGN

#### 3.1 GENERAL

The Masterplan identifies the use of WSUD features to be adopted throughout the project.

Specifically, Raingardens and Tree Pits have been proposed for installation within the Public Realm and in the hardstand areas within the Development lots.

Raingarden Design is to be in accordance with "Water Sensitive Design for Stormwater: Treatment Device Design Guidelines", produced by Wellington Water, December 2019. This requires devices with a minimum area of 2% of the impervious area under treatment. The stormwater design proposed provides a treatment area more than this for the publicly trafficked area for the modified Shelly Bay Road, and the concept satisfies the WSUD provisions for the Development Lots despite building design not yet being finalised. This concept for the Development Lots is based on the current proposed building designs and the development and activities identified in the consented Master Plan.

Figure 3.1.1 below is an extract from the stormwater design plans and illustrates surface and piped flows within each of the catchments extending to the WSUD devices.



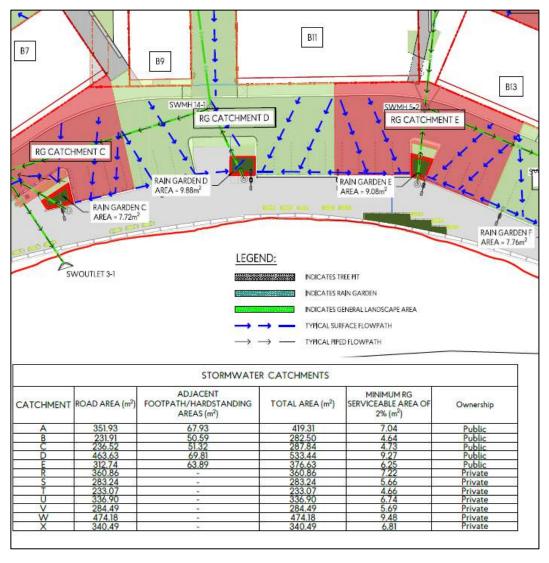


Fig 3.1.1: Raingarden Locations and Stormwater Flowpaths within the North Bay Public Realm

#### 3.2 WSUD DESIGN PROVISIONS

The nature of the WSUD features has evolved in response to the desire to provide suitable water quality treatment focussing on the removal of contaminants, while responding to the constraints of the site which included:

- Limitations of a brownfield development site.
- Flat contours, including in the road reserve and parking areas where limited drainage gradients were possible.
- A desire to minimise earthworks and retain existing landforms as much as possible.

The flat grades and the intended layout with a carparking area adjacent to the existing road has driven the need to propose small individual devices to fit within the carparks and to be fed from localised minor contouring within the relatively small resulting catchments.

The proposed aggregate area of raingardens (excluding tree pits) within the Public Realm (road and esplanade) is 159m<sup>2</sup> for a hardstand area of 5,909m<sup>2</sup>, giving a treatment area of 2.69%. In one low lying location a proprietary "Stormfilter" has been included due to difficulties in providing a raingarden successfully, and this is sized to treat an area of 929.69m<sup>2</sup>.

In addition to treatment via raingardens, tree pits are also proposed that will bring treatment to approximately 4% of the impervious area. Typical tree-pit details are included in Section 3 of the Stormwater Report.



The treated area is limited to the principal trafficked area (both public and private) and Eastern footpaths but does not include much of the seaward pedestrian areas due to practical limitations of existing contours, which tend to fall to the coast. The drawings submitted with this report include illustrations of the defined individual catchments applying to the respective raingardens (refer to plans 1098-01 GW801 – GW807). In all cases the 2% area is achieved for trafficable areas and in all but a few catchments it is achieved for the full area. It is also noted that in most cases areas of sections of the laneway have been included although as noted previously they are not expected to be approved as draining across the road. Generally the capacity is there should approval be granted.

The Public areas have been designed fully designed and proposed details are attached in the stormwater design plans. The effective treatment area provided by each raingarden is in accordance with the Wellington Water "Water Sensitive Design for Stormwater: Treatment Device Design Guideline", 2019 ("WSD Guideline"). Specific design details of the raingardens now include full depth concrete surrounds to maximise the effective areas where possible.

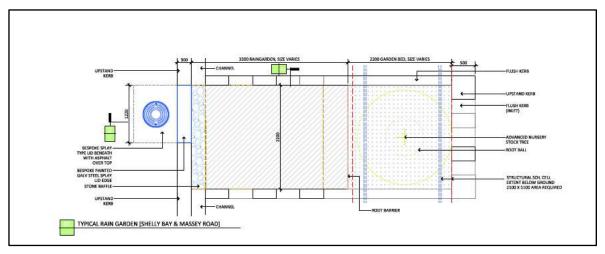
While it is proposed to comply with the minimum provision of 2% for the private Development lots including the commercial area, as noted these have not yet been confirmed due to the designs still being developed. Therefore, it is proposed that the provision of suitable treatment for the private development lots be a condition of the Resource Consent, in accordance with e options provided in the WSD Guideline.

Currently the tree pit area attached to individual raingardens, has not been included due to there not currently being a provision for their inclusion within the WSD Guideline. Because of this the tree pit areas have not been included in achieving the minimum 2% area required. We do understand however that the beneficial use of tree pits is recognised and that their inclusion will be looked on favourably by WWL and will provide effective treatment for an additional area. They would typically increase the effective treatment area by approximately  $4m^2$  per combination device.

With the proposed layout and extent of the devices we are confident that a significant benefit will be achieved due to the associated contaminant removal.

As the Public area raingardens are also designed to provide primary stormwater drainage for events larger than the Water Quality Flow, a Splay Catchpit has been designed to be included to ensure higher flow rates, that could otherwise cause damage to the raingardens, are diverted away from the raingardens to the stormwater network. The required Water Quality raingarden inflow in this instance will be directed to the device utilising a lower-level apron with erosion protection and ensure that the required 200mm minimum storage is provided before diversion occurs.





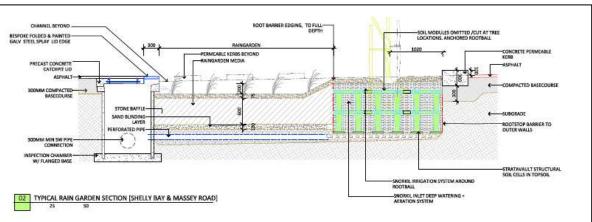


Fig 3.2: Raingarden Details

#### 3.3 SUMMARY AND CONCLUSION

In recognition of the importance of improving the quality of stormwater runoff, and with consideration also of the added importance of the receiving coastal environment, the development has embraced the adoption of Water Sensitive Urban Design to contribute to the improvement of the water quality of fresh and coastal waters.

There are often limitations imposed by working within a "brownfields" site however these have been largely overcome to achieve compliance with the Wellington Water WSD Guideline.

The inclusion of specifically designed bioretention devices such as raingardens, and a commitment to avoiding construction materials and forms of cladding that contribute to contamination by metals such as untreated Copper and Zinc will ensure best practice for stormwater management is achieved.

#### 4.0 LIMITATIONS

#### 4.1 GENERAL

This report has been prepared for Shelly Bay Taikuru and Egmont Dixon in respect of the Shelly Bay Development and its extent is limited to the scope of work agreed between the client and Envelope Engineering Limited. No responsibility is accepted by Envelope Engineering Limited or its directors, servants, agents, staff, or employees for the accuracy of information provided by third parties and/or the use of any part of this report in any other context or for any other purposes.



## **APPENDICES**

#### **APPENDIX 1 CALCULATIONS**

#### STORMWATER PIPE SIZE CALCUATIONS - 1 IN 10 YEAR EVENT

VELOPE					Project N	lame: SHE	LLY BAY							Project I	lo: 1098-0	П		
ormwater Design Chart	Mannings Formula				Location		SAY ROAD	, WELLING	TON					Date: 20	/08/21			
DRAN SECTION	SUB-CATCHMENT DETAILS	SUBCATCHMENT AREA (A)	COEFF OF RUNOFF (C)	EFFECTIVE AREA (CA)	TIME OF CONC (tc)	RANFALL INTENSITY (i) (Including 16% Climate)	EFFECTIVE AREAS (CA)	DESIGN DISCHARGE Q= 278CIA	TOTAL DISCHARGE	PIPELENGTH	PIPE CROSS-SECTIONAL AREA	PPEMATERAL.	'n' FACTOR	PIPE SIZE	ACTUAL SLOPE OF SECTION	VELOCITY	CAPACITY	SPARE CAPACITY
SW LINE 1		ha		ha	min	mm/hr	ha	l/s	l/s	m	m			mm	%		l/s	l/s
To SWMH 1-4	Existing Northern SBR	0.0408	0.95	0.0388	10	84.5	0.0388	9.12										_
SWMH 1-4 to 1-3	RG Catchment A	0.0419	0.95	0.0398	10	84.5	0.0398	9.36			_		_	<b>-</b>	_	_		
	B1	0.0098	0.95	0.0093	10	84.5	0.0093	2.18										
	B3	0.0145	0.95	0.0138	10	84.5	0.0231	5.43	16.97	35.8	0.071	RCRRJ	0.013	300	1.50	2.55	118.4	101.8
SWMH 1-3 to 1-2	Line 2							90.87	107.83	10.0	0.071	RCRRJ	0.013	300	2.00	2.98	136.8	28.9
SWMH 1-2 to OUTLET 1-1	B5	0.0280	0.52	0.0266	10	84.5	0.0266	6.25	114.08	10.5	0.110	RCRRJ	0.013	375	1.50	294	214.7	100.
SWMH I-2 to OUTLET I-I	85	0.0260	0.95	0.0266	10	84.5	0.0266	0.25	114.06	10.5	0.110	KCKKJ	0.013	3/5	1.50	294	214.7	100.
VELOPE					Project N	lame: SHE	LLY BAY							Project I	lo: 1098-0	11		
rmwater Design Chart	Mannings Formula					Date: 20	/08/21											

ENVELOPE					Project N	lame: SHE	LLY BAY								ło: 1098-C	1		
Stormwater Design Chart -	- Mannings Formula				Location:	SHELLY E	AY ROAD	, WELLING	TON					Date: 20.	/08/21			
DRAIN SECTION	SUB-CATCHMENT DETAILS	SUBCATCHMENT AREA(A)	COEFF OF RUNOFF (C)	EFFECTIVE AREA (CA)	TIME OF CONC (tc)	RAINFALL INTENSITY (i) (Including 16% Climate)	EFFCTIVE AREAS (CA)	DESIGN DISCHARGE Q= 2.78CIA	TOTAL DISCHARGE	PIPE LENGTH	PIPE CROSS- SECTIONAL AREA	PIPE MATERAL	'n' FACTOR	PIPE SIZE	ACTUAL SLOPE OF SECTION	VELOCITY	CAPACITY	SPARE CAPACITY
SW LINE 2		ha		ha	min	mm/hr	ha	l/s	l/s	В	ш			mm	%		l/s	l/s
SWMH 2-1 to 1-3	RG Catchment R	0.0361	0.95	0.0343	10	84.5	0.0343	8.05										
	RG Catchment S	0.0283	0.95	0.0269	10	84.5	0.0269	6.32										
	B2	0.0378	0.95	0.0359	10	84.5	0.0359	8.43										
	B4	0.0630	0.95	0.0599	10	84.5	0.0599	14.06										
	El	1.53						54.00	90.87	16.2	0.071	RCRRJ	0.013	300	1.00	2.05	96.7	5.8

Values highlighted yellow have been determined via HEC-HMS analysis

ENVELOPE						lame: SHE									No: 1098-0	и		
Stormwater Design Chart -	Mannings Formula				Location	:SHELLY B	AY ROAD	, WELLING	TON					Date: 20.	/08/21			
DRAIN SECTION	SUB-CATCHMENT DETAILS	SUBCATCHNENT AREA (A)	COEFF OF RUNOFF (C)	EFFECTIVE AREA (CA)	TIME OF CONC (tc)	RANFALL INTENSITY (i) (including 16% Climate)	EFFECTIVE AREAS (CA)	DESIGN DISCHARGE Q= 278CIA	TOTAL DISCHARGE	PIPELENGTH	PIPE CROSS- SECTIONAL AREA	PPE MATERAL	'n' FACTOR	PIPE SIZE	ACTUAL SLOPE OF SECTION	VELOCITY	CAPACITY	SPARE CAPACITY
SW LINE 3		ha		ha	min	mm/hr	ha	l/s	l/s	m	m			mm	%		l/s	l/s
SWMH 3-8 to 3-7	Stream (E2 + E9 + E10)	5.8950						217.00	217.00	10.0	0.110	RCRRJ	0.013	375	7.15	6.75	468.8	251.8
SWMH 3-7 to 3-6	Upper Carpark	0.1101	0.95	0.1046	10	84.5	0.1046	24.58	241.58	15.5	0.110	RCRRJ	0.013	375	5.37	5.80	406.3	164.7
											<u> </u>							
SWMH 3-6 to 3-5	B6	0.0250	0.95	0.0237	10	84.5	0.0237	5.57	247.15	14.7	0.110	RCRRJ	0.013	375	28.81	13.98	941.1	693.9
SWMH 3-5 to 3-4	R8	0.0412	0.95	0.0391	10	845	0.0391	9.19	256.34	14.7	0.110	RCRR I	0.013	375	16.90	10.59	720.8	464.4
344HH 3-3 t0 3-4	00	0.0412	0.73	0.0371	10	84.5	0.0371	7.17	200.34	14.7	0.110	NCNNJ	0.013	3/0	10.70	10.37	120.0	404.4
SWMH 3-4 to 3-3	R4	0.0630	0.95	0.0599	10	84.5	0.0599	14.06	270.40	10.5	0.159	RCRR I	0.013	450	100	265	285.1	14.7
								700										
SWMH 3-3 to 3-2	RG Catchment T	0.0233	0.95	0.0221	10	84.5	0.0221	5.20										
	RG Catchment U	0.0337	0.95	0.0320	10	84.5	0.0320	7.52	283.12	22.0	0.159	RCRRJ	0.013	450	1.59	3.40	359.5	76.4
SWMH 3-2 to OUTLET 3-1	RG Catchment B	0.0283	0.95	0.0268	10	84.5	0.0268	6.30										
	RG Catchment C	0.0288	0.95	0.0273	10	84.5	0.0273	6.42										
	B5	0.0280	0.95	0.0266	10	84.5	0.0266	6.25										
	B7	0.0248	0.95	0.0236	10	84.5	0.0236	5.53										
	Line 14							31.62	339.25	17.0	0.283	RCRRJ	0.013	600	0.70	2.63	513.7	174.5

NVELOPE						lame: SHE									ło: 1098-0	1		
ormwater Design Chart -	Mannings Formula				Location	:SHELLY B	AY ROAD	, WELLING	TON					Date: 20,	/08/21			
DRAN SECTION	SUB-CATCHMENT DETAILS	SUBCATCHMENT AREA(A)	COEFF OF RUNOFF (C)	EFFECTIVE AREA (CA)	TIME OF CONC (tc)	RAINFALL INTENSITY (i) (Including 16% Climate)	EFFECTIVE AREAS (CA)	DESIGN DISCHARGE Q= 2.78CIA	TOTAL DISCHARGE	PIPE LENGTH	PIPE CROSS- SECTIONAL AREA	PIPE MATERAL	'n' FACTOR	PIPE SIZE	ACTUAL SLOPE OF SECTION	VELOCITY	CAPACITY	SPARE CAPACITY
SW LINE 4		ha		ha	min	mm/hr	ha	l/s	l/s	Е	Е			mm	%		l/s	1/s
To SWMH 4-3	RG Catchment Y	0.0516	0.95	0.0490	10	84.5	0.0490	11.52										
	RG Catchment Z	0.0388	0.95	0.0368	10	84.5	0.0368	8.65										
	B14	0.0504	0.95	0.0479	10	84.5	0.0479	11.25										
	B16	0.0378	0.95	0.0359	10	84.5	0.0359	8.44										
	E4	0.2458						10.00	49.85									
SWMH 4-3 to 4-2	B15 B17	0.0392	0.95	0.0372	10	84.5 84.5	0.0372	8.74 11.27										_
	B17 R19	0.0305	0.95	0.0262	10	84.5	0.0460	6.16		21.5	0.071	RCRRJ	0.013	300	3.00	3.70	167.5	91.5
	BIA	0.0276	0.95	0.0262	10	04.5	0.0262	0.10	76.02	21.5	0.071	KCKKJ	0.013	300	3.00	3.70	107.5	YIJ
SWMH 4-2 to OUTLET 4-1	RG Catchment G	0.0233	0.95	0.0221	10	845	0.0221	5.20										
	RG Catchment H	0.0337	0.95	0.0320	10	84.5	0.0320	7.52										
	Line 5		2.70			1		134.99										
	Line 15							52.17	275.90	18.2	0.283	RCRRJ	0.013	600	0.80	2.82	549.2	273

ENVELOPE						lame: SHE									No: 1098-0	)1		
Stormwater Design Chart	- Mannings Formula				Location	:SHELLY B	AY ROAD	, WELLING	TON					Date: 20	/08/21			
DRAIN SECTION	SUB-CATCHMENT DETAILS	SUBCATCHMENT AREA(A)	COEFF OF RUNOFF (C)	EFFECTIVE AREA (CA)	TIME OF CONC (16)	RAINFALL INTENSITY (i) (hcluding 16% Climate)	EFFECTIVE AREAS (CA)	DESIGN DISCHARGE Q= 2.78CIA	TOTAL DISCHARGE	HIBNETENGIH	PIPE CROSS- SECTIONAL AREA	PIPE MATERIAL	'n' FACTOR	PIPE SIZE	ACTUAL SLOPE OF SECTION	VELOCITY	CAPACITY	SPARE CAPACITY
SW LINE 5		ha		ha	min	mm/hr	ha	l/s	l/s	m	m			mm	%		l/s	l/s
To SWMH 5-3	RG Catchment X	0.0340	0.95	0.0323	10	84.5	0.0323	7.60										
	B10	0.0629	0.95	0.0598	10	84.5	0.0598	14.04										
	B12	0.0376	0.95	0.0357	10	84.5	0.0357	8.38	30.02									
SWMH 5-3 to 5-2	No additional flow							0.00	30.02	23.0	0.071	RCRRJ	0.013	300	1.00	2.05	96.7	66.7
SWMH 5-2 to 5-1	RG Catchment E	0.0377	0.95	0.0358	10	84.5	0.0358	8.41										
	B11	0.0364	0.95	0.0346	10	84.5	0.0346	8.12										
	B13	0.0248	0.95	0.0236	10	84.5	0.0236	5.54	52.09	24.0	0.071	RCRRJ	0.013	300	1.00	2.05	96.7	44.6
SWMH 5-1 to 4-2	RG Catchment F	0.0422	0.95	0.0401	10	84.5	0.0401	9.42						-				
3441-111 3-1 (0 4-2	B13	0.0248	0.95	0.0236	10	84.5	0.0236	5.54						<b>—</b>		<u> </u>		
	B15	0.0392	0.95	0.0372	10	84.5	0.0372	8.74										
	Line 6							59.20	134.99	35.0	0.159	RCRRJ	0.013	450	1.00	2.65	285.1	150.1
	1														L	L		
	1																	

NVELOPE						lame: SHE									lo: 1098-0	1		
tormwater Design Chart - Ma	nnings Formula				Location:	SHELLY B	AY ROAD	, WELLING	TON					Date: 20,	/08/21			
DRAIN SECTION	SUB-CATCHMENT DETAILS	SUBCATCHMENT AREA(A)	COEFF OF RUNOFF (C)	EFFECTIVE AREA (CA)	TIME OF CONC (tc)	RAINFALL INTENSITY (i) (induding 16% Climate)	EFECTIVE AREAS (CA)	DESIGN DISCHARGE Q= 2.78CIA	TOTAL DISCHARGE	PIPE LENGTH	PPE CROSS- SECTIONAL AREA	PIPE MATERIAL	'n' FACTOR	PIPE SIZE	ACTUAL SLOPE OF SECTION	VELOCITY	CAPACITY	SPARE CAPACITY
SW LINE 6		ha		ha	min	mm/hr	ha	l/s	l/s	ε	Е			mm	%		l/s	l/s
SWMH 6-1 to 5-1 Ram	np to Upper Carparl	0.0384	0.95	0.0365	10	84.5	0.0365	8.58										
	B12	0.0376	0.95	0.0357	10	84.5	0.0357	8.38										
	B14	0.0504	0.95	0.0479	10	84.5	0.0479	11.25										
	E3	0.7503						31.00	59.20	50.0	0.110	RCRRJ	0.013	375	1.00	2.36	175.3	116.1

NVELOPE						lame: SHE		, WELLING	TO!!						No: 1098-0	1		
Stormwater Design Chart -	Mannings Formula		_			SHELLY E								Date: 20	/08/21			_
DRAIN SECTION	SUB-CATCHMENT DETALS	SUBCATCHMENT AREA(A)	COEFF OF RUNOFF (C)	EFFECTIVE AREA (CA)	TIME OF CONC (tc)	RAINFALL INTENSITY (i) (Including 16% Climate)	EFFECTIVE AREAS (CA)	DESIGN DISCHARGE Q= 2.78CIA	TOTAL DISCHARGE	PPELENGTH	PIPE CROSS- SECTIONAL AREA	PIPE MATERIAL	'n' FACTOR	PIPE SIZE	ACTUAL SLOPE OF SECTION	VELOCITY	CAPACITY	
SW LINE 7		ha		ha	min	mm/hr	ha	l/s	l/s	m	m			mm	%		l/s	I
																		ļ
SWMH 7-6 to 7-5	RG Catchment K	0.0930	0.95	0.0883	10	84.5	0.0883	20.75										ļ
	B20	0.1886	0.95	0.1792	10	84.5	0.1792	42.09							-			Ł
	B21 B22	0.0099	0.95	0.0094	10	84.5 84.5	0.0094	2.21		_	-	<b>—</b>	_	-	-	<b>—</b>		ł
	B22 B23	0.0755	0.95	0.0717	10	84.5	0.0717	6.13										ł
	E6	0.0275	0.93	0.0261	10	84.5	0.0261	8.20	96.22	33.5	0.159	RCRRJ	0.013	450	0.50	1.83	201.6	ł
	E0	0.2006		-				8.20	90.22	33.5	0.159	RCRRJ	0.013	400	0.50	1.03	201.6	ł
SWMH 7-5 to 7-4	B27	0.0143	0.95	0.0136	10	84.5	0.0136	3.20	99.42	34.5	0.216	RCRRJ	0.013	525	0.40	1.79	272.0	٠
34414117-3107-4	027	0.0143	0.73	0.0130	10	04.0	0.0130	3.20	77.42	34.3	0.210	NCNNJ	0.013	020	0.40	1.77	212.0	t
SWMH 7-4 to 7-3	RG Catchment L	0.0561	0.95	0.0533	10	84.5	0.0533	12.52										t
3441417 4107 0	Line 8	0.0001	0.70	0.0000	- 10	04.0	0.0000	34.29	146.22	31.5	0.283	RCRRJ	0.013	600	0.30	1.66	336.3	t
									1-10.22								-	t
SWMH 7-3 to 7-2	RG Catchment M	0.0550	0.95	0.0522	10	84.5	0.0522	12.27										t
	B29	0.0362	0.95	0.0344	10	84.5	0.0344	8.07	166.57	19.2	0.283	RCRRJ	0.013	600	0.30	1.66	336.3	T
																		Τ
SWMH 7-2 to OUTLET 7-1	RG Catchment N	0.0214	0.95	0.0203	10	84.5	0.0203	4.77										I
	RG Catchment O	0.0369	0.95	0.0351	10	84.5	0.0351	8.24										L
	B31	0.0269	0.95	0.0255	10	84.5	0.0255	6.00										L
	B33	0.0335	0.95	0.0318	10	84.5	0.0318	7.48										Ļ
	B36	0.0168	0.95	0.0159	10	84.5	0.0159	3.74										Ļ
	Line 9							59.56	256.37	48.5	0.358	RCRRJ	0.013	675	0.30	1.79	460.4	Į.
														_	-			Ł
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							•				•	•						۰

ENVELOPE					Project N	lame: SHE	LLY BAY								lo: 1098-0	1		
Stormwater Design Chart -	Mannings Formula				Location	:SHELLY B	AY ROAD	, WELLING	TON					Date: 20.	/08/21			
DRAIN SECTION	SUB-CATCHMENT DETAILS	SUBCATCHMENT AREA (A)	COEFF OF RUNOFF (C)	EFFCTIVE AREA (CA)	TIME OF CONC (tc)	RAINFALL INTENSITY (i) (including 16% Climate)	EFFECTIVE AREAS (CA)	DESIGN DISCHARGE Q= 2.78CIA	TOTAL DISCHARGE	PIPE LENGTH	PIPE CROSS- SECTIONAL AREA	PIPE MATERIAL	'h' FACTOR	PIPE SIZE	ACTUAL SLOPE OF SECTION	VELOCITY	CAPACITY	SPARE CAPACITY
SW LINE 8		ha		ha	min	mm/hr	ha	l/s	l/s	m	m			mm	%		l/s	l/s
SWMH 8-2 to 8-1	B24	0.0100	0.95	0.0095	10	84.5	0.0095	2.24										
	B25	0.0100	0.95	0.0095	10	84.5	0.0095	2.23										
	E7	0.0880						3.50	7.97	21.0	0.071	RCRRJ	0.013	300	1.29	2.35	109.8	101.9
SWMH 8-1 to 7-4	RG Catchment AA	0.0551	0.95	0.0523	10	84.5	0.0523	12.29										
	B28	0.0629	0.95	0.0597	10	84.5	0.0597	14.03	34.29	23.2	0.071	RCRRJ	0.013	300	3.50	4.02	180.9	146.6

NVELOPE						lame: SHE									lo: 1098-0	1		
Stormwater Design Chart -	Mannings Formula				Location	SHELLY E	AY ROAD	, WELLING	TON					Date: 20,	/08/21			
DRAIN SECTION	SUB-CATCHMENT DETAILS	SUBCATCHMENT AREA (A)	COEFF OF RUNOFF (C)	EFECTIVE AREA (CA)	TIME OF CONC (tc)	RAINFALL INTENSITY (i) (including 16% Climate)	EFFECTIVE AREAS (CA)	DESIGN DISCHARGE Q= 2.78CIA	TOTAL DISCHARGE	PIPELENGTH	PIPE CROSS- SECTIONAL AREA	PIPE MATERIAL	'h' FACTOR	PIPE SIZE	ACTUAL SLOPE OF SECTION	VELOCITY	CAPACITY	SPARE CAPACITY
SW LINE 9		ha		ha	min	mm/hr	ha	l/s	l/s	m	m			mm	%		l/s	l/s
		0.0017	0.05	0.0005	10	0.15	0.0005			_								
SWMH 9-2 to 9-1	RG Catchment AC RG Catchment AD	0.0247	0.95	0.0235	10	84.5 84.5	0.0235	5.52 6.21		_								
	RG Catchment AD R34	0.0278	0.95	0.0264			0.0264	3.03		_					_			
					10	84.5												
	B35	0.0312	0.95	0.0296	10	84.5	0.0296	6.95	21.72	33.0	0.071	RCRRJ	0.013	300	1.00	2.05	96.7	75.0
SWMH 9-1 to 7-2	RG Catchment AB	0.0328	0.95	0.0312	10	84.5	0.0312	7.33										
	B30	0.0475	0.95	0.0451	10	84.5	0.0451	10.59										
	B32	0.0503	0.95	0.0478	10	84.5	0.0478	11.23										
	E8	0.2090						8.70	59.56	23.5	0.071	RCRRJ	0.013	300	4.80	4.75	211.9	152.3

ENVELOPE						lame: SHE									lo: 1098-0	1		
Stormwater Design Chart -	Mannings Formula				Location		AY ROAD	, WELLING	TON					Date: 20.	/08/21			
DRAINSECTION	SUB-CATCHMENT DETAILS	SUBCATCHMENT AREA (A)	COEFF OF RUNOFF (C)	EFFECTIVE AREA (CA)	TIME OF CONC (tc)	RAINFALL INTENSITY (i) (including 16% Climate)	EFFECTIVE AREAS (C.A.)	DESIGN DISCHARGE Q= 2.78CIA	TOTAL DISCHARGE	PIPE LENGTH	PIPE CROSS- SECTIONAL AREA	PIPE MATERIAL	'n' FACTOR	PIPE SIZE	ACTUAL SLOPE OF SECTION	VELOCITY	CAPACITY	SPARE CAPACITY
SW LINE 10		ha		ha	min	mm/hr	ha	l/s	l/s	m	m			mm	%		l/s	l/s
SWMH 10-4 to 10-3	RG Catchment P	0.0320	0.95	0.0304	10	84.5	0.0304	7.15										
	RG Catchment Q	0.0173	0.95	0.0164	10	84.5	0.0164	3.86										
	B35	0.0312	0.95	0.0296	10	84.5	0.0296	6.95										
	B37	0.0083	0.95	0.0078	10	84.5	0.0078	1.84	19.80	41.5	0.071	RCRRJ	0.013	300	0.50	1.40	68.4	48.6
SWMH 10-3 to 10-2	R38	0.0096	0.95	0.0092	10	84.5	0.0092	2.15										
3 VVMH 10-3 to 10-2	SBR 1	0.0098	0.95	0.0092	10	84.5	0.0092	11.41								_		
	SBR 1	0.0511	0.95	0.0486	10	84.5	0.0486	10.66					-	-		-		
	Line 11	0.0476	0.93	0.0454	10	04.3	0.0454	268.00	312.03	46.0	0.283	RCRR I	0.013	600	0.74	2.71	528.2	216.2
	Life II							200.00	312.03	40.0	0.203	NCKKJ	0.013	800	0.74	2.71	320.2	210.2
SWMH 10-2 to OUTLET 10-1	B39	0.0120	0.95	0.0114	10	84.5	0.0114	2.68		-			<b>-</b>	<b>-</b>		<b>-</b>		
34414110210001221101	B40	0.0128	0.95	0.0121	10	84.5	0.0121	2.85										
	B41	0.0158	0.95	0.0150	10	84.5	0.0150	3.52										
	B42	0.0106	0.95	0.0101	10	84.5	0.0101	2.37										
	B43	0.0114	0.95	0.0108	10	84.5	0.0108	2.55										
	B44	0.0109	0.95	0.0103	10	84.5	0.0103	2.43										
	Line 12							165.37	493.79	9.5	0.358	RCRRJ	0.013	675	1.00	3.43	840.6	346.8
										L								
	I	1	1	1	ı	ı	ı			1	ı	ı	1	1	ı	ı		

ELD ST ODE					la		1137 B 437							h	1 1000 0			
ENVELOPE Stormwater Design Chart -	Mannings Formula												Project No: 1098-01 Date: 20/08/21					
DRAIN SECTION	SUBCATCHMENT DETAILS	SUBCATCHMENT AREA (A)	COEFF OF RUNOFF (C)	EFECTIVE AREA (CA)	TIME OF CONC (tc)	RAINFALL INTENSITY (i) (including 16% Climate)	EFFECTIVE AREAS (CA)	DESIGN DISCHARGE Q= 2.78CIA	TOTAL DISCHARGE	PIPELENGTH	PIPE CROSS- SECTIONAL AREA	PIPE MATERIAL	h' FACTOR	PIPE SIZE	ACTUAL SLOPE OF SECTION	VELOCITY	CAPACITY	SPARE CAPACITY
SW LINE TI		ha		ha	min	mm/hr	ha	l/s	l/s	Е	m			mm	%		l/s	l/s
SWMH 11-3 to 11-2 to 11-1 to 10-3	E12	4.2200						176.00						(minimu	m values)			
	E13	3.7930						92.00	268.00		0.110	RCRRJ	0.013	375	7.58	6.96	482.7	214.7
										-	0.159	RCRRJ	0.013	450	3.00	4.77	493.8	225.8

ENVELOPE						lame: SHE									lo: 1098-0	1		
Stormwater Design Chart -	Mannings Formula				Location	SHELLY B	AY ROAD	, WELLING	TON					Date: 20,	/08/21			
DRAIN SECTION	SUB-CATCHMENT DETAILS	SUBCATCHMENT AREA (A)	COEFF OF RUNOFF (C)	EFFECTIVE AREA (CA)	TIME OF CONC (tc)	RAINFALL INTENSITY (i) (Including 16% Climate)	0	DESIGN DISCHARGE Q= 2.78CIA	TOTAL DISCHARGE	PIPELENGTH	PPE CROSS- SECTIONAL AREA	PIPE MATERIAL	'n' FACTOR	PIPE SIZE	ACTUAL SLOPE OF SECTION	VELOCITY	CAPACITY	SPARE CAPACITY
SW LINE 12		ha		ha	min	mm/hr	ha	l/s	l/s	m	m			mm	%		l/s	l/s
SWMH 12-2 to 12-1	B40	0.0128	0.95	0.0121	10	84.5	0.0121	2.85										
	E14	3.7930						159.00	161.85	12.7	0.071	RCRRJ	0.013	300	20.00	10.08	432.5	270.6
SWMH 12-1 to 10-2	B41	0.0158	0.95	0.0150	10	84.5	0.0150	3.52	165.37	22.5	0.110	RCRRJ	0.013	375	5.00	5.58	392.1	226.7

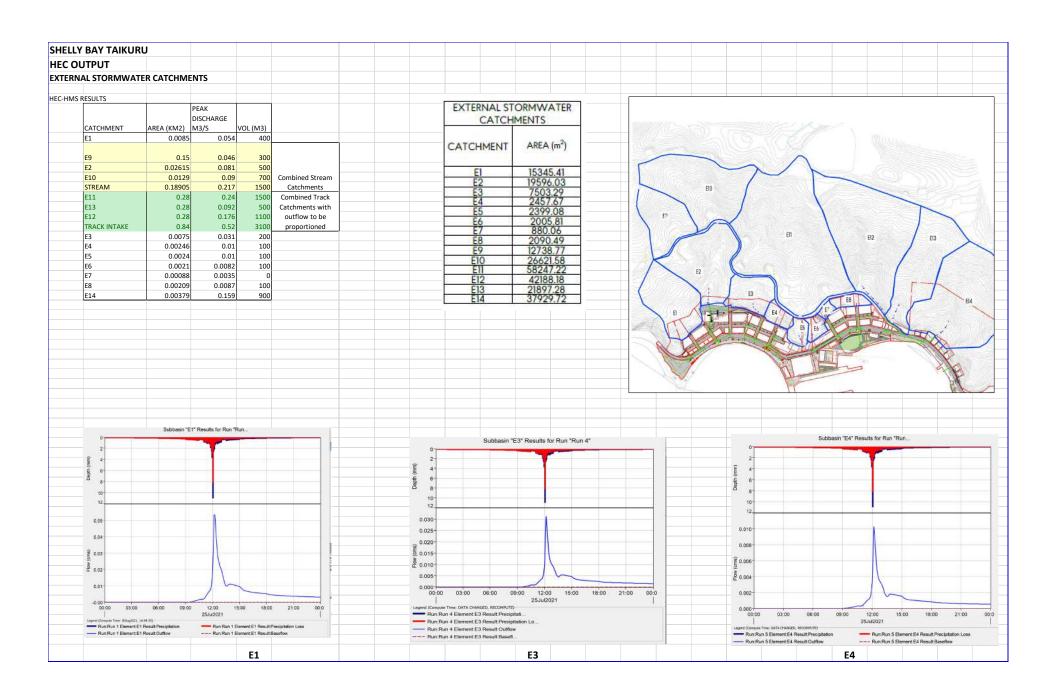
ENVELOPE CLASS	M					lame: SHE		MELLINIC	TON						lo: 1098-0	1		
Stormwater Design Chart -	mannings rormula				Location:SHELLY BAY ROAD, WELLINGTON								Date: 20/08/21					
DRAIN SECTION	SUB-CATCHMENT DETAILS	SUBCATCHMENT AREA (A)	COEFF OF RUNOFF (C)	EFFECTIVE AREA (CA)	TIME OF CONC (tc)	RAINFALL INTENSITY (i) (Including 16% Climate)	0	DESIGN DISCHARGE Q= 2.78CIA	TOTAL DISCHARGE	PIPE LENGTH	PPE CROSS- SECTIONAL AREA	PIPE MATERIAL	'n' FACTOR	PIPE SIZE	ACTUAL SLOPE OF SECTION	VELOCITY	CAPACITY	SPARE CAPACITY
SW LINE 13		ha		ha	min	mm/hr	ha	l/s	l/s	m	m			mm	%		l/s	l/s
SWMH 13-2 to OUTLET 13-1	B45	0.0151	0.95	0.0144	10	84.5	0.0144	3.37										
	B46	0.0118	0.95	0.0112	10	84.5	0.0112	2.63										
	SBR3	0.0417	0.95	0.0396	10	84.5	0.0396	9.30	15.30	11.5	0.110	RCRRJ	0.013	375	1.84	3.28	237.8	222.5

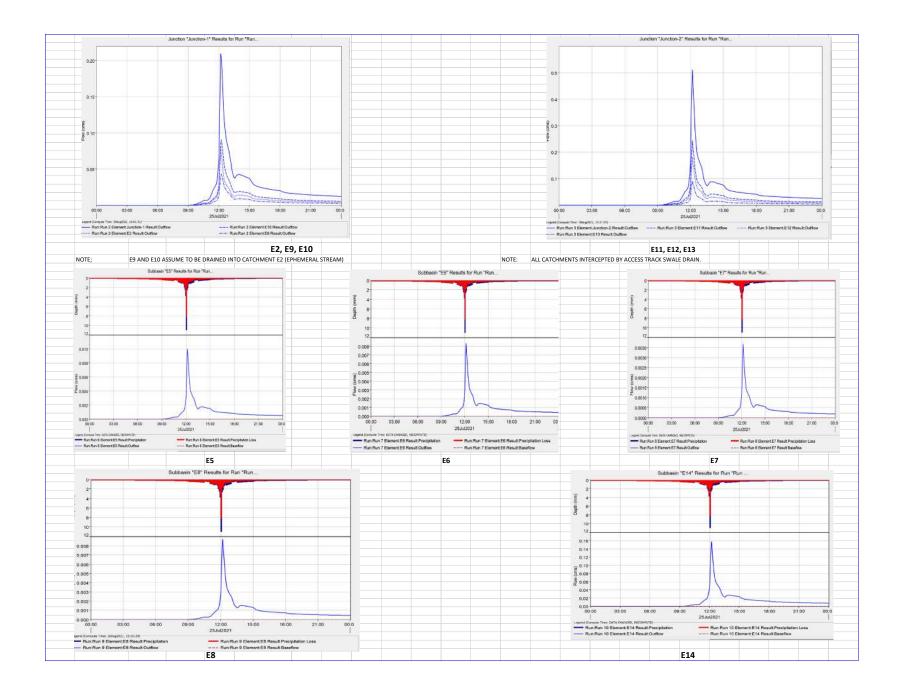
ENVELOPE Stormwater Design Chart -	Manatasa Fassaula					lame: SHE		), WELLING	TON					Project N Date: 20	lo: 1098-0	1		
DISAIN SECTION	SUB-CATCH/MENT DETAILS	SUBCATCHMENT AREA (A)	COEFF OF RUNOFF (C)	BFECTIVE AREA (CA)	TME OF CONC (tc)	RAINFALL INTENSITY (i) (including 16% Climate)	EFFECTIVE AREAS (CA)	DESIGN DISCHARGE	TOTAL DISCHARGE	PIPE LENGTH	PPE CROSS- SECTIONAL AREA	PIPE MATERIAL	'n' FACTOR	PIPE SIZE	ACTUAL SLOPE OF SECTION	VELOCITY	CAPACITY	SPARE CAPACITY
SW LINE 14		ha		ha	min	mm/hr	ha	l/s	l/s	m	m			mm	%		l/s	l/s
SWMH 14-1 to 13-2	RG Catchment D	0.0533	0.95	0.0507	10	84.5	0.0507	11.90										
	RG Catchment V	0.0284	0.95	0.0270	10	84.5	0.0270	6.35										
	RG Catchment W	0.0474	0.95	0.0450	10	84.5	0.0450	10.58										
	B9	0.0125	0.95	0.0119	10	84.5	0.0119	2.79	31.62	33.6	0.071	RCRRJ	0.013	300	0.89	1.92	91.2	59.6

ENVELOPE						lame: SHE									lo: 1098-0	A .			
Stormwater Design Chart	- Mannings Formula				Location	SHELLY E	BAY ROAD	), WELLING	TON					Date: 20/08/21					
DRAIN SECTION	SUB-CATCHMENT DETAILS	SUBCATCHMENT AREA (A)	COEFF OF RUNO FF (C)	EFFECTIVE AREA (CA)	TIME OF CONC (10)	RANFALL INTENSITY (i) (Including 16% Climate)	EFFECTIVE AREAS (CA)	DESIGN DISCHARGE Q= 2.78CIA	TOTAL DISCHARGE	PPELENGTH	PIPE CROSS- SECTIONAL AREA	PIPE MATERIAL	'n' FACTOR	PIPE SIZE	ACTUAL SLOPE OF SECTION	VELOCITY	CAPACITY	SPARE CAPACITY	
SW LINE 15		ha		ha	min	mm/hr	ha	l/s	l/s	Е	m			mm	%		l/s	l/s	
SWMH 15-1 to 4-2	RG Catchment I	0.0270	0.95	0.0256	10	84.5	0.0256	6.02											
	RG Catchment J	0.0587	0.95	0.0558	10	84.5	0.0558	13.10											
	B17	0.0505	0.95	0.0480	10	84.5	0.0480	11.27											
	B18	0.0252	0.95	0.0239	10	84.5	0.0239	5.61											
	B19	0.0276	0.95	0.0262	10	84.5	0.0262	6.16											
	E5	0.23991						10.00	52.17	25.1	0.110	RCRRJ	0.013	375	1.00	2.36	175.3	123.2	
	1		1			l .									l .				

#### Rainfall intensities (mm/hr) :: RCP6.0 for the period 2081-2100

API	AEP	10m	20m
1.58	0.633	48.8	33.5
2	0.500	53.8	36,9
5	0.200	71.2	48.7
10	0.100	84.5	57.7
20	0.050	98.5	67.1
30	0.033	107	72.9
40	0.025	113	77.0
50	0.020	118	80.4
60	0.017	122	83.1
80	0.012	129	87.5
100	0.010	134	90.9
250	0.004	155	105





## **DRAWINGS**

#### **APPENDIX 2 DRAWINGS**

## **ENVELOPE**

#### LAND STRUCTURE MANAGE

CLIENT

THE WELLINGTON COMPANY

PROJECT:

SHELLY BAY
SHELLY BAY ROAD
WELLINGTON

PALN SET:

CIVIL ENGINEERING DRAWINGS

ISSUE

**GWRC CONSENT ISSUE** 

DATE

6th SEPTEMBER 2021

REFERENCE:

1098-01



LOCATION PLAN SCALE A1 - 1:5000, A3 - 1:10000

## **ENVELOPE**

#### LAND STRUCTURE MANAGE

CLIENT

THE WELLINGTON COMPANY

PROJECT:

SHELLY BAY
SHELLY BAY ROAD
WELLINGTON

PLAN SET

CIVIL ENGINEERING DRAWINGS

ISSUE

**GWRC CONSENT ISSUE** 

DATE:

6th SEPTEMBER 2021

L1, 125 VINCENT STREET AUCKLAND CITY 1010 PO BOX 68946 NEWTON 1141 ENVELOPE ENGINEERING

REFERENCE:

1098-01

DRAWING	NAME
1098-01-200	EXISTING CONTOUR PLAN - OVERALL LAYOUT
1098-01-201	EXISTING CONTOUR PLAN - SHEET 1 OF 3
1098-01-202	EXISTING CONTOUR PLAN - SHEET 2 OF 3
1098-01-203	EXISTING CONTOUR PLAN - SHEET 3 OF 3
1098-01-210	PROPOSED CONTOUR PLAN - OVERALL LAYOUT
1098-01-211	PROPOSED CONTOUR PLAN - SHEET 1 OF 3
1098-01-212	PROPOSED CONTOUR PLAN - SHEET 2 OF 3
1098-01-213	PROPOSED CONTOUR PLAN - SHEET 3 OF 3
1098-01-220	PROPOSED CUT/FILL PLAN - OVERALL LAYOUT
1098-01-221	PROPOSED CUT/FILL PLAN - SHEET 1 OF 3
1098-01-222	PROPOSED CUT/FILL PLAN - SHEET 2 OF 3
1098-01-223	PROPOSED CUT/FILL PLAN - SHEET 3 OF 3
1098-01-230	PROPOSED EROSION AND SEDIMENT CONTROL PLAN - OVERALL
1098-01-231	PROPOSED EROSION AND SEDIMENT CONTROL PLAN - SHEET 1 OF 3
1098-01-232	PROPOSED EROSION AND SEDIMENT CONTROL PLAN - SHEET 2 OF 3
1098-01-233	PROPOSED EROSION AND SEDIMENT CONTROL PLAN - SHEET 3 OF 3
1098-01-235	EROSION AND SEDIMENT CONTROL DETAILS - SHEET 1 OF 3
1098-01-236	EROSION AND SEDIMENT CONTROL DETAILS - SHEET 2 OF 3
1098-01-237	EROSION AND SEDIMENT CONTROL DETAILS - SHEET 3 OF 3
1098-01-400	PROPOSED PUBLIC DRAINAGE PLANS - OVERALL LAYOUT
1098-01-401	PROPOSED PUBLIC DRAINAGE PLANS - NORTH BAY
1098-01-402	PROPOSED PUBLIC DRAINAGE PLANS - SOUTH BAY
1098-01-403	PROPOSED PUBLIC DRAINAGE PLANS - SHEET 1 OF 13
1098-01-404	PROPOSED PUBLIC DRAINAGE PLANS - SHEET 2 OF 13
1098-01-405	PROPOSED PUBLIC DRAINAGE PLANS - SHEET 3 OF 13
1098-01-406	PROPOSED PUBLIC DRAINAGE PLANS - SHEET 4 OF 13
1098-01-407	PROPOSED PUBLIC DRAINAGE PLANS - SHEET 5 OF 13
1098-01-408	PROPOSED PUBLIC DRAINAGE PLANS - SHEET 6 OF 13
1098-01-409	PROPOSED PUBLIC DRAINAGE PLANS - SHEET 7 OF 13
1098-01-410	PROPOSED PUBLIC DRAINAGE PLANS - SHEET 8 OF 13
1098-01-411	PROPOSED PUBLIC DRAINAGE PLANS - SHEET 9 OF 13
1098-01-412	PROPOSED PUBLIC DRAINAGE PLANS - SHEET 10 OF 13
1098-01-413	PROPOSED PUBLIC DRAINAGE PLANS - SHEET 10 OF 13
1098-01-414	PROPOSED PUBLIC DRAINAGE PLANS - SHEET 12 OF 13
1098-01-415	PROPOSED PUBLIC DRAINAGE PLANS - SHEET 13 OF 13
1098-01-413	PROPOSED FORMWATER LONG-SECTIONS - SHEET 1 OF 5
1098-01-421	PROPOSED STORMWATER LONG-SECTIONS - SHEET 2 OF 5
1098-01-421	PROPOSED STORMWATER LONG-SECTIONS - SHEET 3 OF 5
1098-01-423	PROPOSED STORMWATER LONG-SECTIONS - SHEET 4 OF 5
1098-01-424	PROPOSED STORMWATER LONG-SECTIONS - SHEET 5 OF 5
1098-01-424	WASTEWATER PUMP STATION DETAILS - SHEET 1 OF 4
1098-01-490	WASTEWATER POMP STATION DETAILS - SHEET 2 OF 4  WASTEWATER PUMP STATION DETAILS - SHEET 2 OF 4
1098-01-491	WASTEWATER FUMP STATION DETAILS - SHEET 3 OF 4
1098-01-493	WASTEWATER PUMP STATION DETAILS - SHEET 4 OF 4 STORMWATER OUTLET DETAILS
1098-01-494	STORMWATER OUTLET DETAILS  STORMWATER INLET DETAILS
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1098-01-496	MANHOLE AND SPLAY CATCHPIT DETAILS
1098-01-497	PIPE BEDDING AND TRENCHING DETAILS
1098-01-498	STORMWATER 360 - STORMFILTER DETAILS
1098-01-499	RAINGARDEN DETAILS

DRAWING INDEX

1098-01-GW800	INTERNAL STORMWATER CATCHMENTS - OVERALL LAYOUT
1098-01-GW801	INTERNAL STORMWATER CATCHMENTS - SHEET 1 OF 7
1098-01-GW802	INTERNAL STORMWATER CATCHMENTS - SHEET 2 OF 7
1098-01-GW803	INTERNAL STORMWATER CATCHMENTS - SHEET 3 OF 7
1098-01-GW804	INTERNAL STORMWATER CATCHMENTS - SHEET 4 OF 7
1098-01-GW805	INTERNAL STORMWATER CATCHMENTS - SHEET 5 OF 7
1098-01-GW806	INTERNAL STORMWATER CATCHMENTS - SHEET 6 OF 7
1098-01-GW807	INTERNAL STORMWATER CATCHMENTS - SHEET 7 OF 7
1098-01-900	EXTERNAL STORMWATER CATCHMENTS

#### NOTES:

- 1. CONTOURS SHOWN ARE PROPOSED FINISHED GROUND LEVELS AND ARE SHOWN AT 0.5m INTERVALS.
- 2. LEVELS ARE IN TERMS OF WELLINGTON VERTICAL DATUM 1953 ORIGIN RM II SO 31470 RL 3.05m
- 3. ALL WORKS TO COMPLY WITH THE WELLINGTON CITY COUNCIL CODE OF LAND DEVELOPMENT
- ALL PUBLIC DRAINAGE DESIGN IS IN ACCORDANCE WITH WELLINGTON WATER REGIONAL STANDARDS FOR WATER SERVICES, DETAILS SHOWN ARE SUBJECT TO FURTHER DESIGN DEVELOPMENT AND FINAL APPROVALS FROM WELLINGTON WATER AND WCC.
- ALL PRIVATE DRAINAGE INCLUDING RAINGARDENS WITHIN THE PRIVATE ACCESSWAY WILL BE COVERED UNDER A SEPARATE BUILDING CONSENT APPLICATION.
- ALL STORMWATER AND WASTEWATER CONNECTIONS ARE SHOWN INDICATIVELY. THE LOCATION, DIMENSIONS AND NUMBER OF CONNECTIONS WILL BE CONFIRMED AT THE BUILDING CONSENT STAGE
- 7. ALL STORMWATER PIPES TO BE RCRRJ CLASS 2 UNLESS SHOWN OTHERWISE.
- 8. ALL MANHOLES TO BE 1050mmØ UNLESS SHOWN OTHERWISE. REFER TO 1098-01-420 TO 434.
- 9. ALL STORMWATER SINGLE SUMP LEADS TO BE RCRRJ CLASS 4 DN 225.
- 10. ALL STORMWATER DOUBLE SUMP LEADS TO BE RCRRJ CLASS 4 DN 300.



LEGEND:

 INDICATIVE BUILDING FOOTPRINT NDICATES STORMWATER - PROPOSED — INDICATES STORMWATER - PRIVATE INDICATES WASTEWATER - PROPOSED — INDICATES WASTEWATER - PRIVATE INDICATES WASTEWATER - RISING MAIN INDICATES BACK-ENTRY CATCHPIT

INDICATES PRIVATE RAIN GARDEN AREA

DRAIN — INDICATES TRENCH DRAIN (ACO OR SIMILAR)

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REVIS	SIONS:		
REV	NOTES	BY	DATE
R1	RESOURCE CONSENT ISSUE	PJ	13-09-2016
R2	FOR DEVELOPMENT AGREEMENT (INTERNAL)	JW	30/06/2021
R3	FOR DEVELOPMENT AGREEMENT	JW	09/07/2021
El	FOR ENGINEERING APPROVAL	JW	03/09/21

SHELLY BAY TAIKURU LIMITED SHELLY BAY WELLINGTON

PUBLIC DRAINAGE PLANS **OVERALL LAYOUT** 



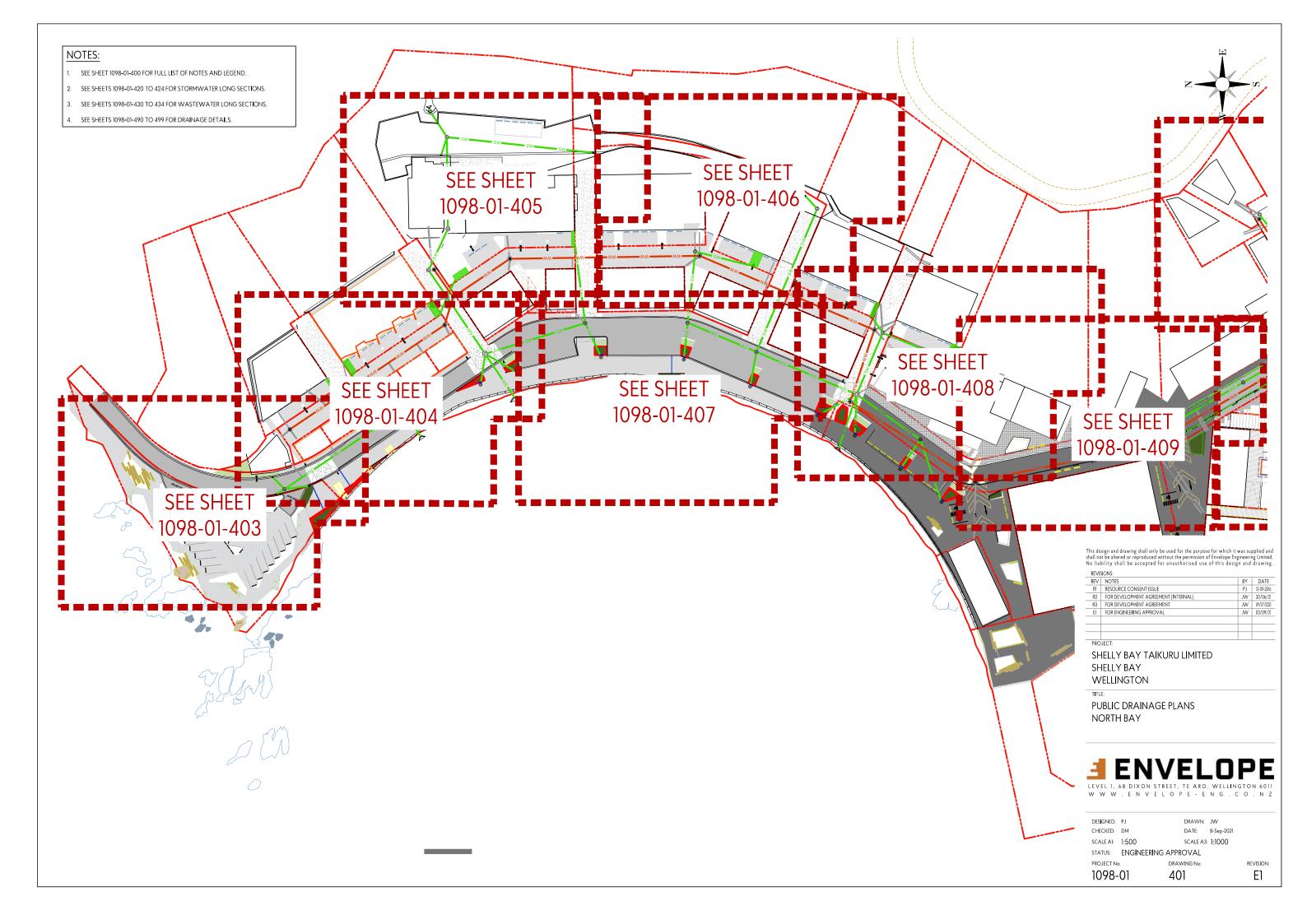
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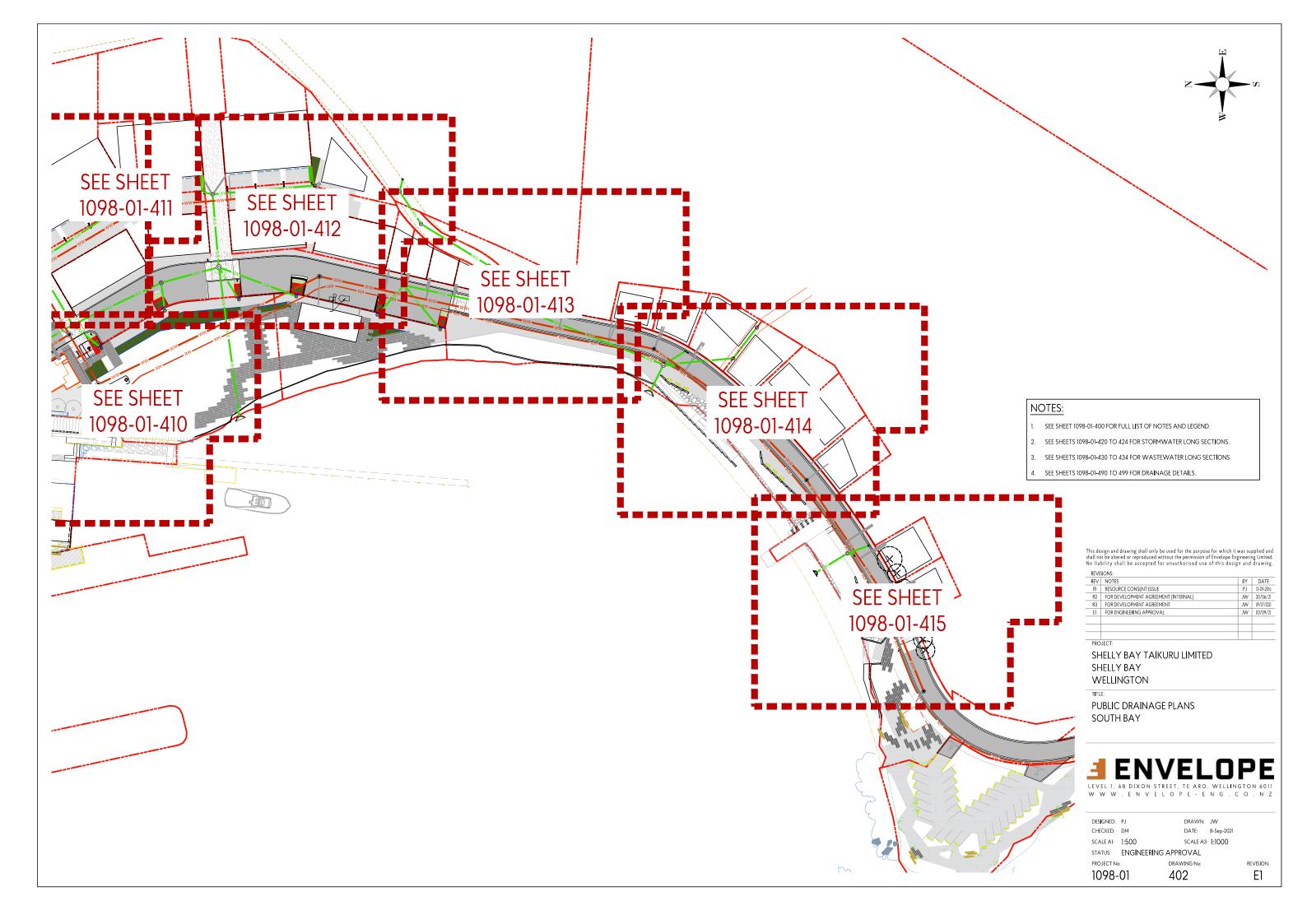
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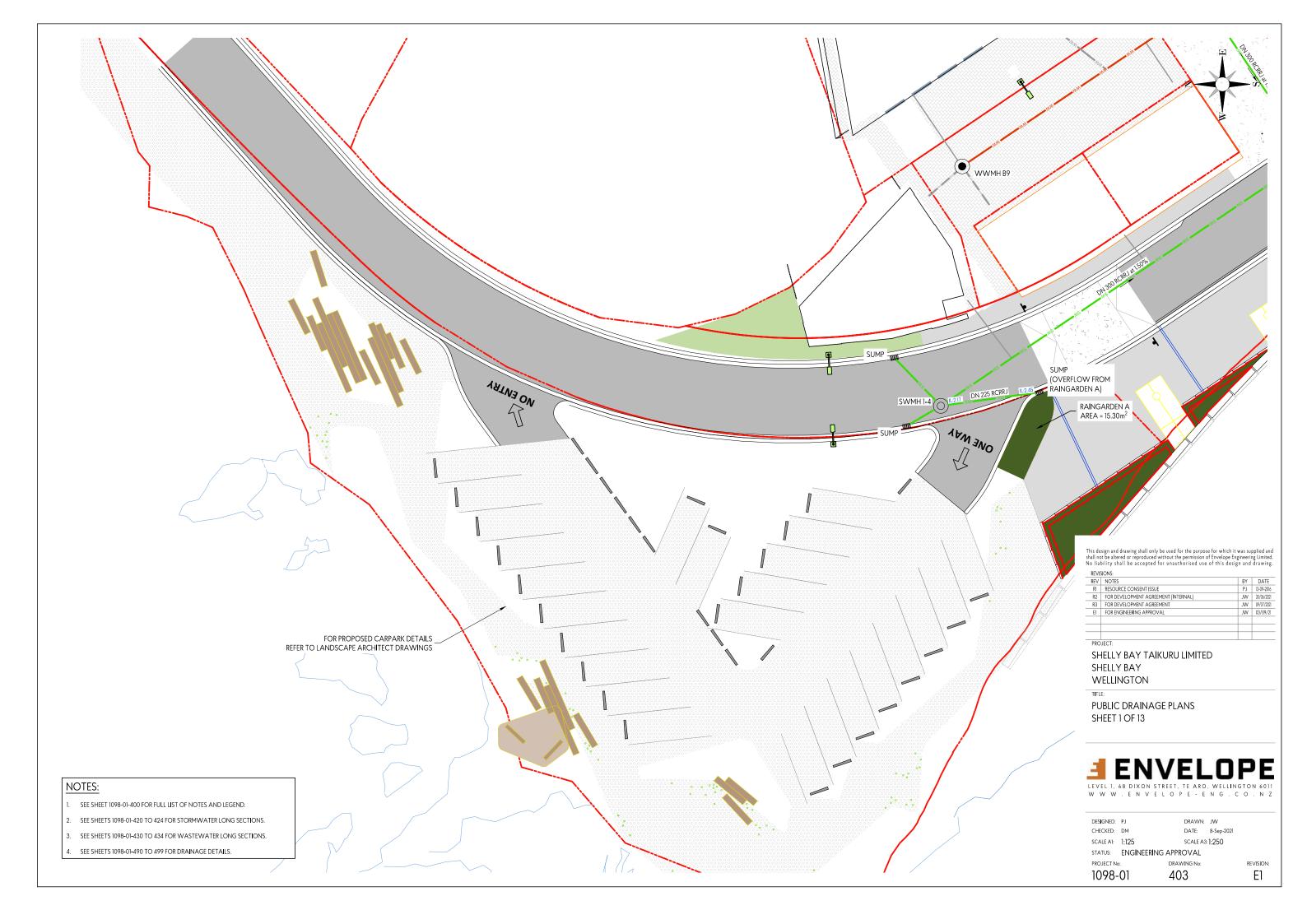
1098-01

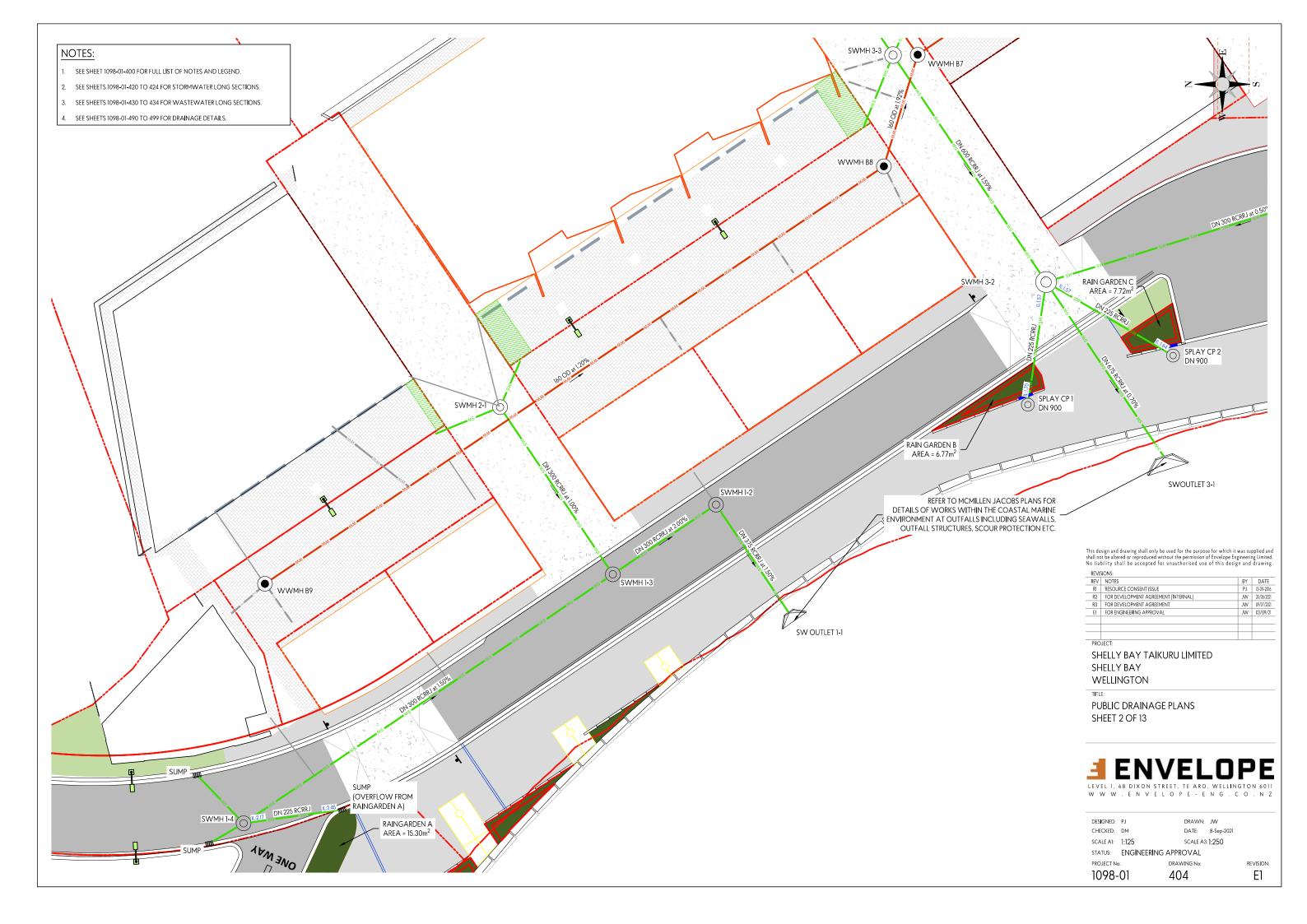
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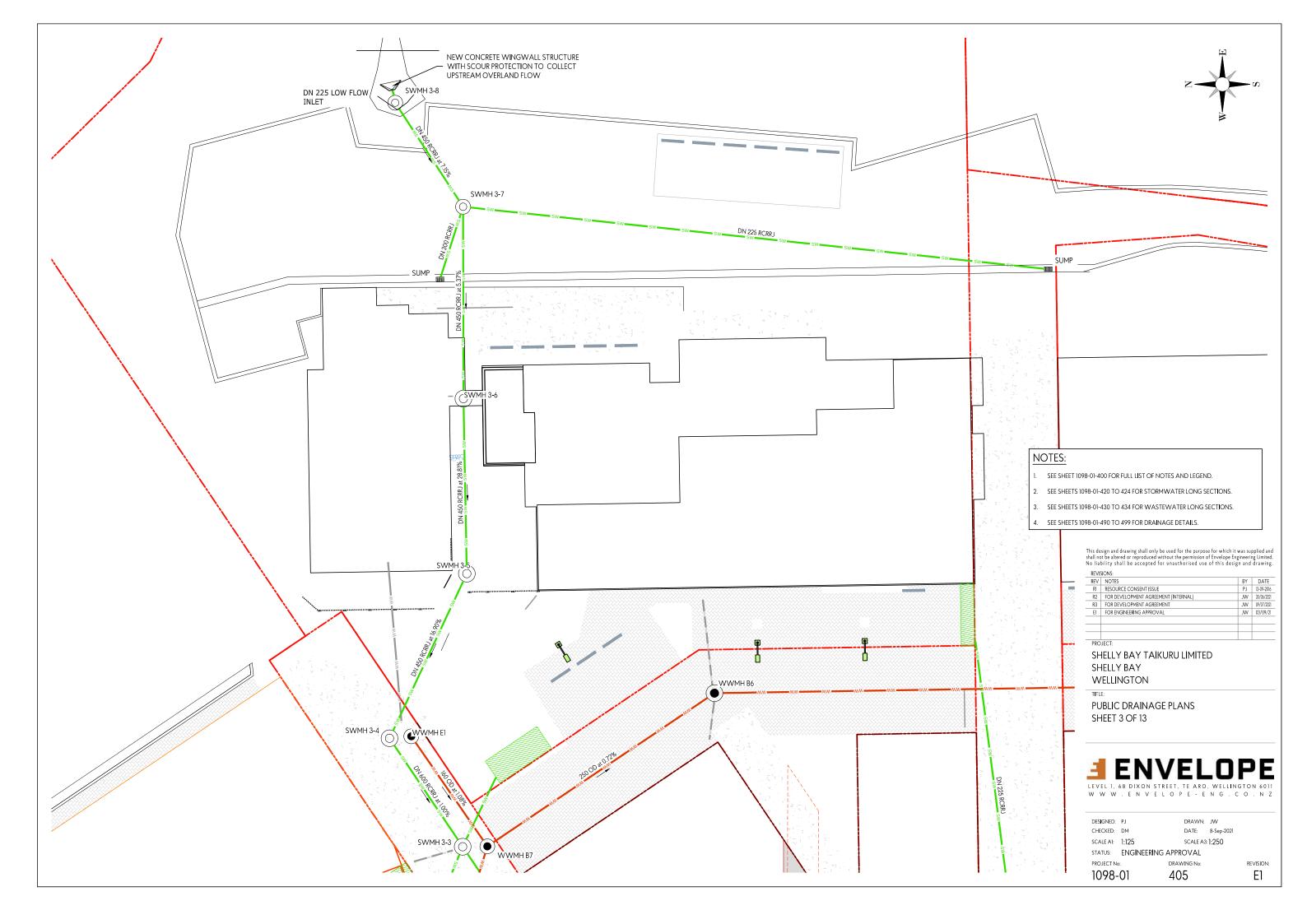
REVISION: E1







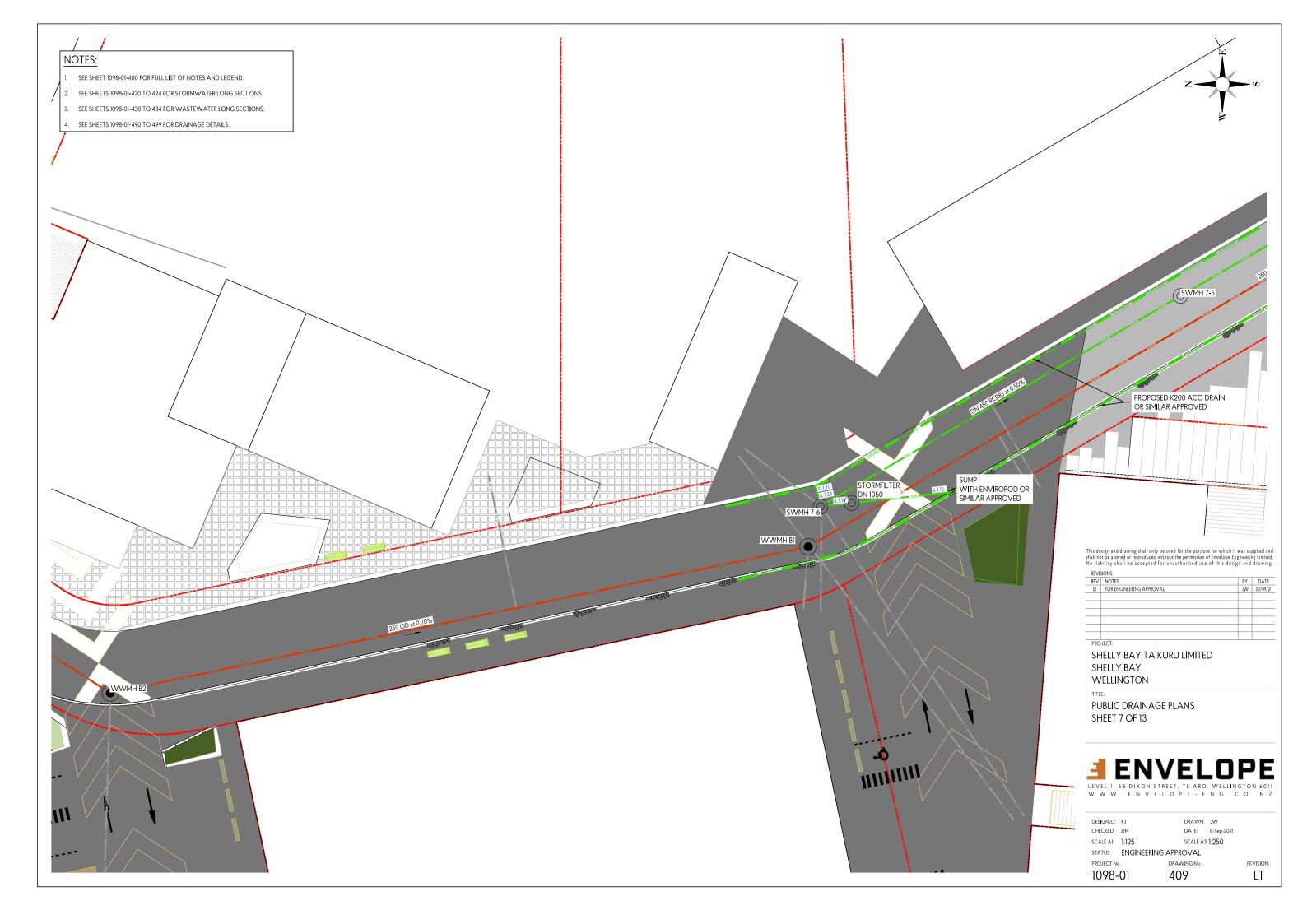




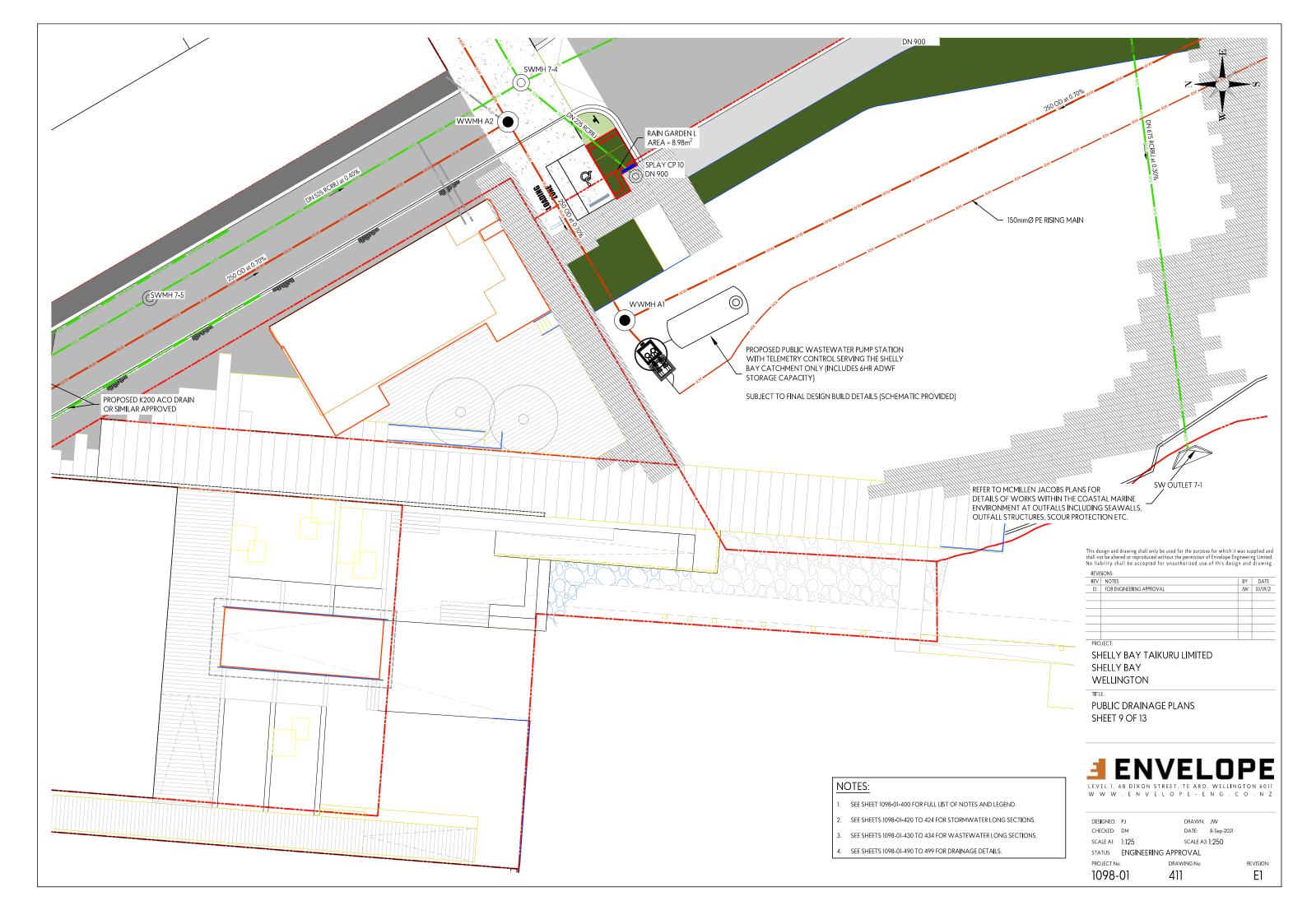






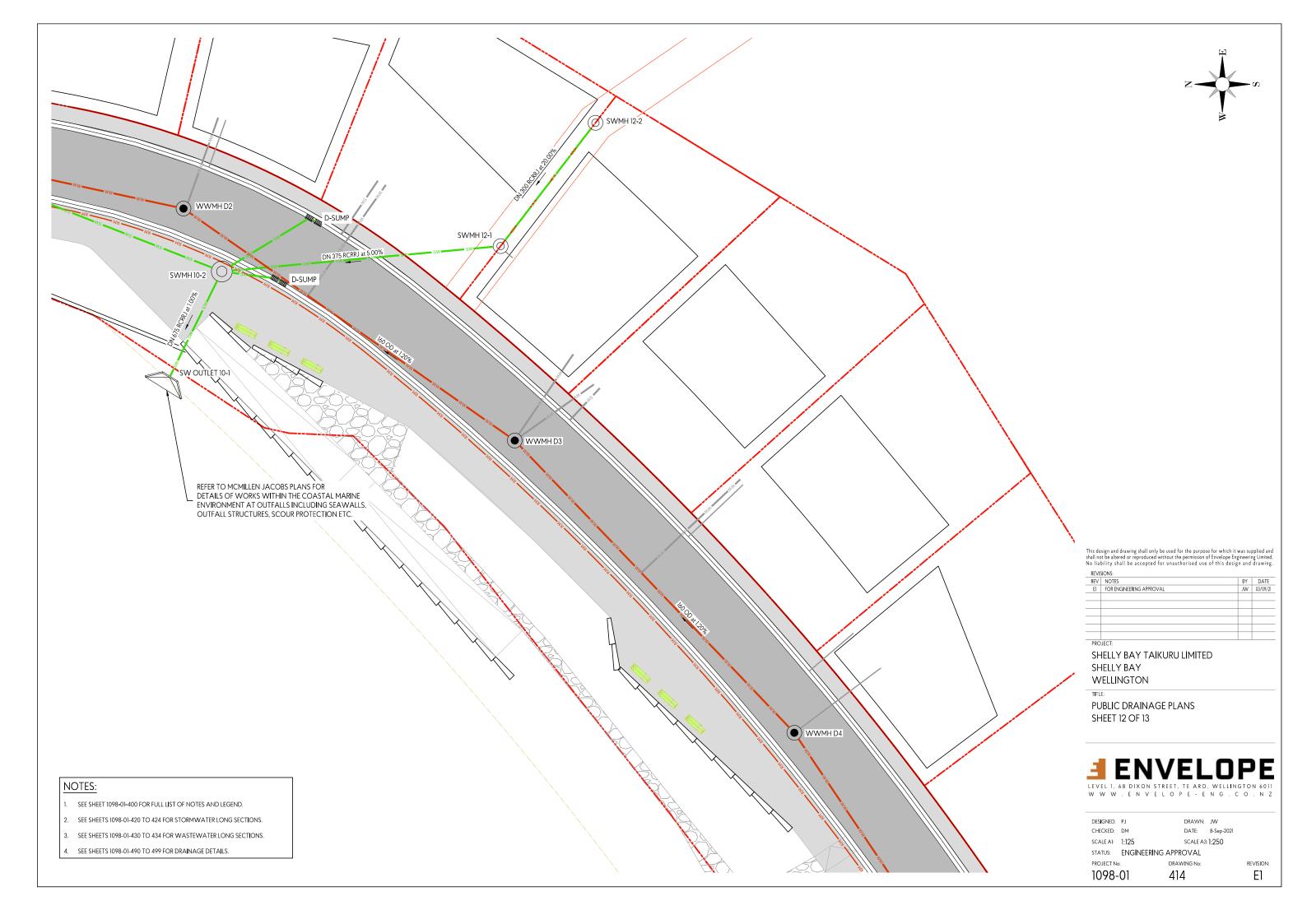
















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		v	
REVIS	SIONS:		
REV	NOTES	BY	DAT
E1	FOR ENGINEERING APPROVAL	JW	27/08
PRO	JECT:		

SHELLY BAY TAIKURU LIMITED SHELLY BAY WELLINGTON

TITLE:

PUBLIC DRAINAGE PLANS SHEET 13 OF 13



V W W . E N V E L O P E - E N G . C O . N Z

 DESIGNED:
 P J
 DRAWN:
 JW

 CHECKED:
 DM
 DATE:
 8-5ep-2021

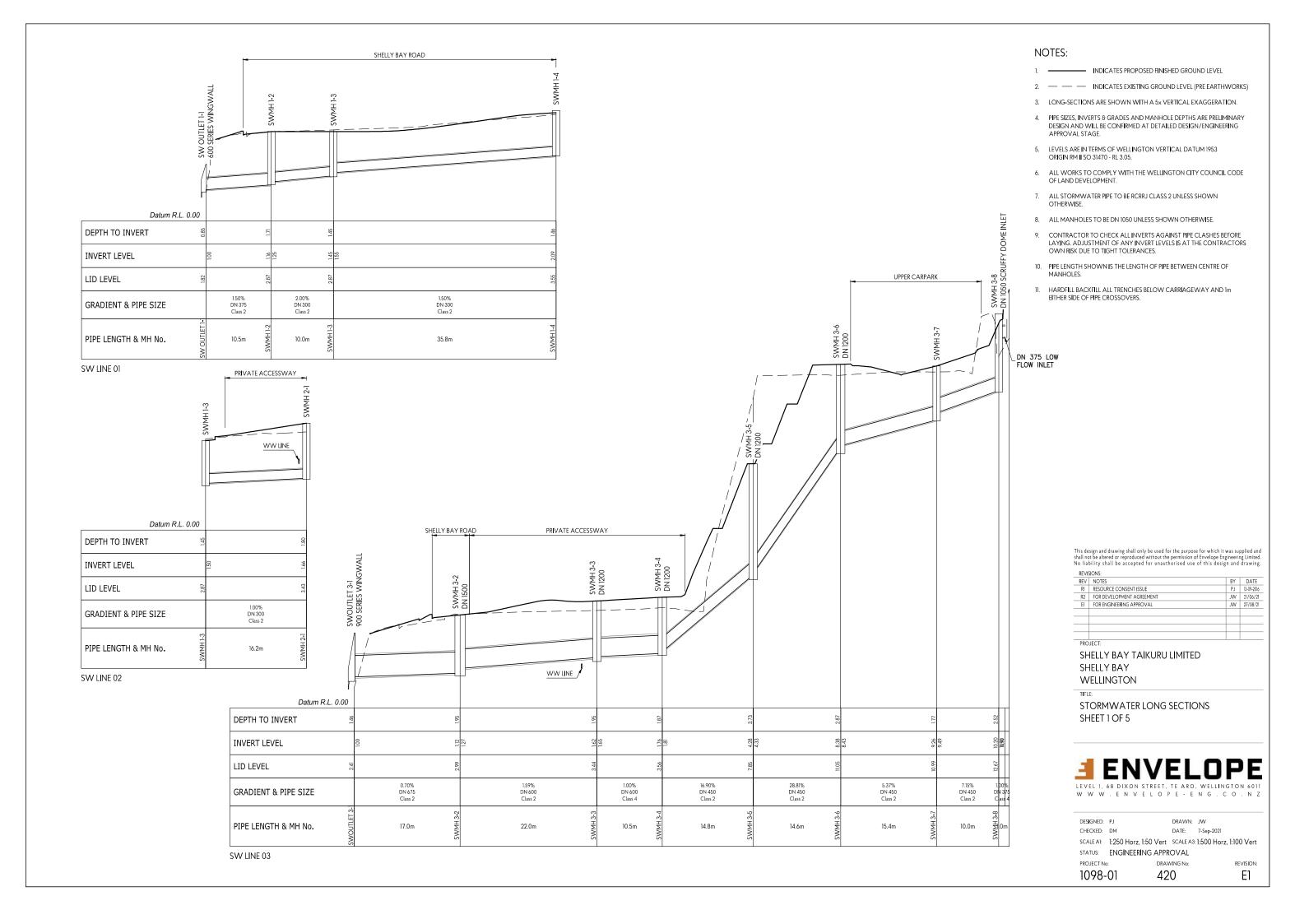
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 1:125
 SCALE A3: 1:250

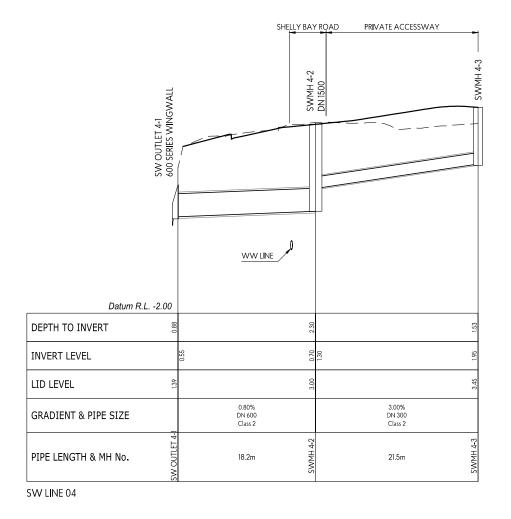
 STATUS:
 FNGINEERING APPROVAL

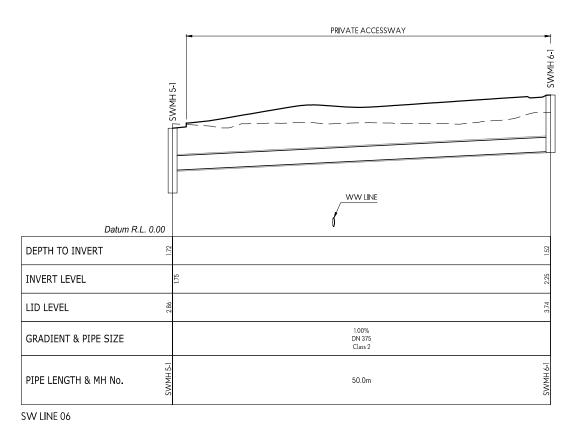
STATUS: ENGINEERING APPROVAL
PROJECT No: DRAWING No:

1098-01 415

revision:







| SHELLY BAY ROAD | PRIVATE ACCESSWAY | STATE | STATE

SW LINE 05

#### NOTES:

- 1. INDICATES PROPOSED FINISHED GROUND LEVEL
- 2. — INDICATES EXISTING GROUND LEVEL (PRE EARTHWORKS)
- 3. LONG-SECTIONS ARE SHOWN WITH A 5x VERTICAL EXAGGERATION.
- PIPE SIZES, INVERTS & GRADES AND MANHOLE DEPTHS ARE PRELIMINARY DESIGN AND WILL BE CONFIRMED AT DETAILED DESIGN/ENGINEERING APPROVAL STAGE.
- 5. LEVELS ARE IN TERMS OF WELLINGTON VERTICAL DATUM 1953 ORIGIN RM II SO 31470 - RL 3.05.
- 6. ALL WORKS TO COMPLY WITH THE WELLINGTON CITY COUNCIL CODE OF LAND DEVELOPMENT.
- 7. ALL STORMWATER PIPE TO BE RCRRJ CLASS 2 UNLESS SHOWN OTHERWISE.
- 8. ALL MANHOLES TO BE DN 1050 UNLESS SHOWN OTHERWISE.
- CONTRACTOR TO CHECK ALL INVERTS AGAINST PIPE CLASHES BEFORE LAYING. ADJUSTMENT OF ANY INVERT LEVELS IS AT THE CONTRACTORS OWN RISK DUE TO TIGHT TOLERANCES.
- 10. PIPE LENGTH SHOWN IS THE LENGTH OF PIPE BETWEEN CENTRE OF
- 11. HARDFILL BACKFILL ALL TRENCHES BELOW CARRIAGEWAY AND 1m EITHER SIDE OF PIPE CROSSOVERS.

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REVIS	SIONS:		
REV	NOTES	BY	DATE
RI	RESOURCE CONSENT ISSUE	PJ	13-09-2016
R2	FOR DEVELOPMENT AGREEMENT	JW	21/06/21
E1	FOR ENGINEERING APPROVAL	JW	27/08/21
DDO	IECT.		

SHELLY BAY TAIKURU LIMITED SHELLY BAY WELLINGTON

TITLE

STORMWATER LONG SECTIONS
SHEET 2 OF 5



DESIGNED: PJ DRAWN: JW
CHECKED: DM DATE: 7-Sep-2021

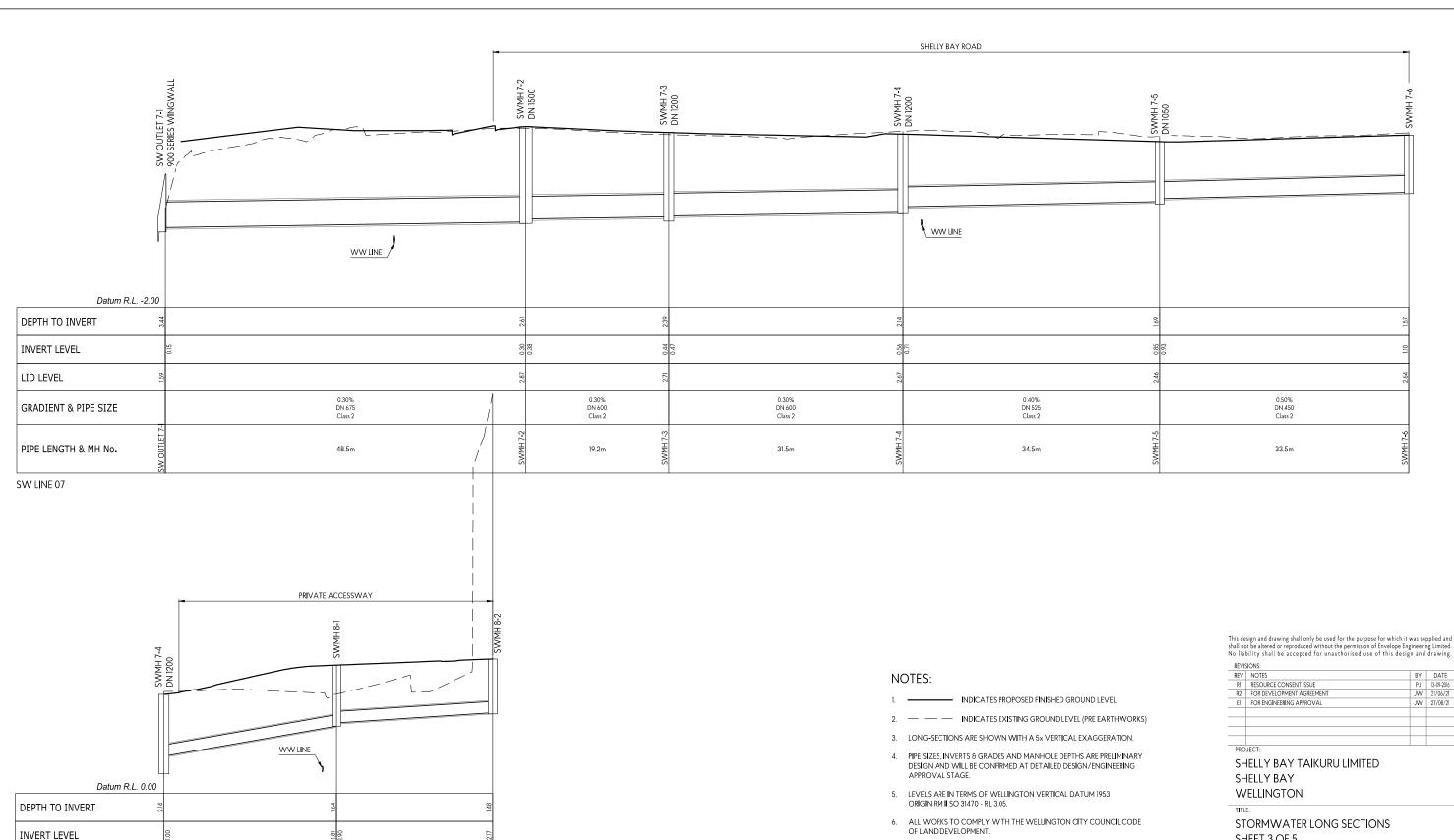
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STATUS: ENGINEERING APPROVAL
PROJECT No: DRAWING No:

1098-01 421

E1

REVISION:



LID LEVEL

SW LINE 08

**GRADIENT & PIPE SIZE** 

PIPE LENGTH & MH No.

3.50% DN 300 Class 2

23.2m

1.29% DN 300 Class 2

21.0m

SHEET 3 OF 5

7. ALL STORMWATER PIPE TO BE RCRRJ CLASS 2 UNLESS SHOWN

8. ALL MANHOLES TO BE DN 1050 UNLESS SHOWN OTHERWISE.

OWN RISK DUE TO TIGHT TOLERANCES.

9. CONTRACTOR TO CHECK ALL INVERTS AGAINST PIPE CLASHES BEFORE LAYING. ADJUSTMENT OF ANY INVERT LEVELS IS AT THE CONTRACTORS

10. PIPE LENGTH SHOWN IS THE LENGTH OF PIPE BETWEEN CENTRE OF

11. HARDFILL BACKFILL ALL TRENCHES BELOW CARRIAGEWAY AND 1m EITHER SIDE OF PIPE CROSSOVERS.

OTHERWISE.

MANHOLES.



DESIGNED: PJ DRAWN: JW CHECKED: DM DATE: 7-Sep-2021

SCALE AI: 1:250 Horz, 1:50 Vert SCALE A3: 1:500 Horz, 1:100 Vert STATUS: ENGINEERING APPROVAL

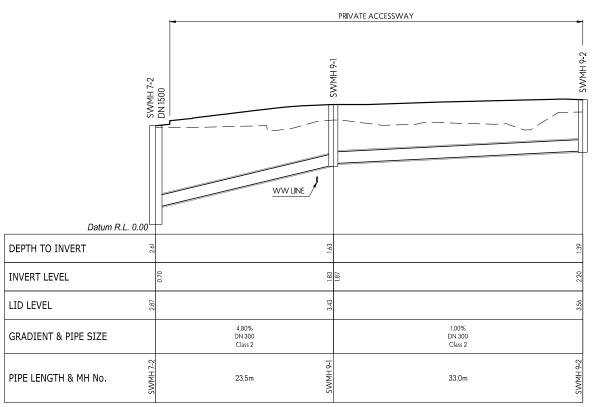
PROJECT No: DRAWING No:

1098-01

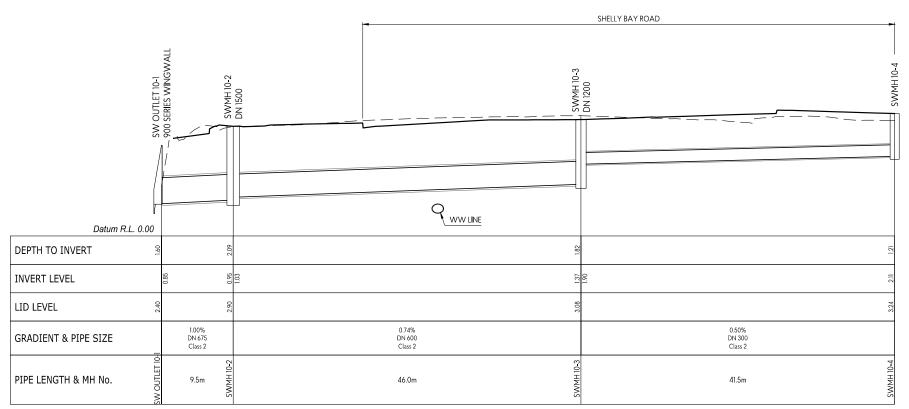
REVISION:

E1

422



SW LINE 09



SW LINE 10

- INDICATES PROPOSED FINISHED GROUND LEVEL
- 2. — INDICATES EXISTING GROUND LEVEL (PRE EARTHWORKS)
- 3. LONG-SECTIONS ARE SHOWN WITH A 5x VERTICAL EXAGGERATION.
- 4. PIPE SIZES, INVERTS & GRADES AND MANHOLE DEPTHS ARE PRELIMINARY DESIGN AND WILL BE CONFIRMED AT DETAILED DESIGN/ENGINEERING APPROVAL STAGE.
- 5. LEVELS ARE IN TERMS OF WELLINGTON VERTICAL DATUM 1953 ORIGIN RM II SO 31470 - RL 3.05.
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- 10. PIPE LENGTH SHOWN IS THE LENGTH OF PIPE BETWEEN CENTRE OF
- 11. HARDFILL BACKFILL ALL TRENCHES BELOW CARRIAGEWAY AND 1m EITHER SIDE OF PIPE CROSSOVERS.

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REV	NOTES	BY	DATE
RI	RESOURCE CONSENT ISSUE	PJ	13-09-2016
R2	FOR DEVELOPMENT AGREEMENT	JW	21/06/2
El	FOR ENGINEERING APPROVAL	JW	27/08/2

SHELLY BAY TAIKURU LIMITED SHELLY BAY WELLINGTON

STORMWATER LONG SECTIONS SHEET 4 OF 5



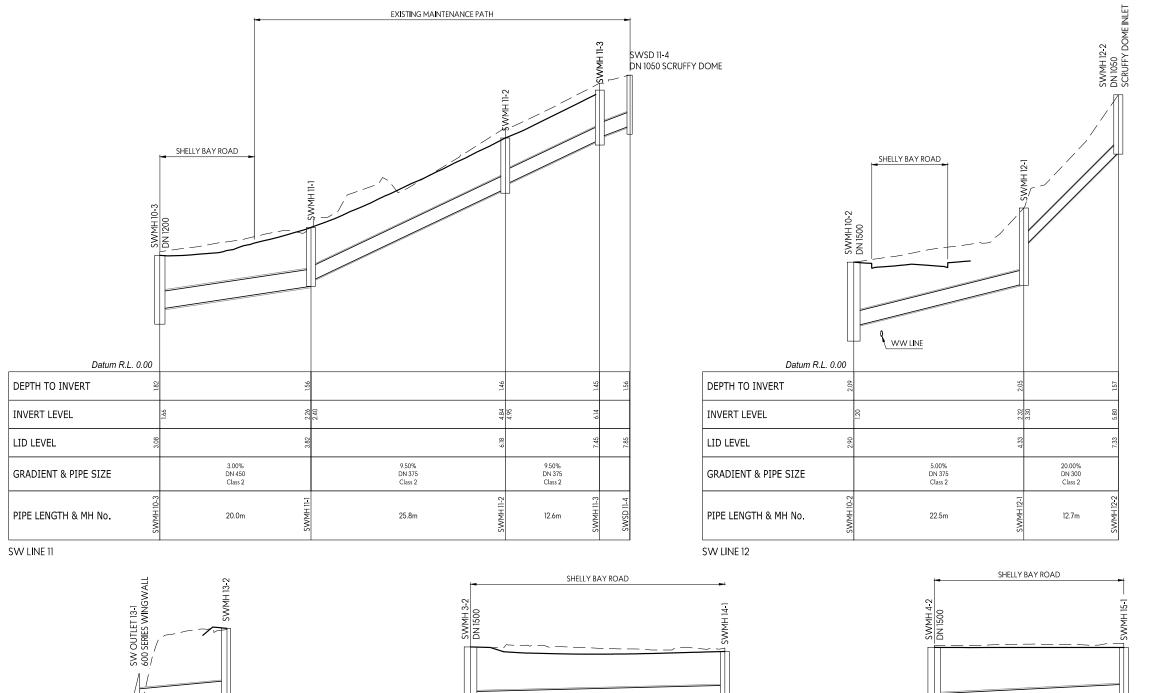
DESIGNED: PJ DRAWN: JW CHECKED: DM DATE: 7-Sep-2021

SCALE AI: 1:250 Horz, 1:50 Vert SCALE A3: 1:500 Horz, 1:100 Vert

STATUS: ENGINEERING APPROVAL PROJECT No: DRAWING No:

423 1098-01

REVISION: E1



0.50% DN 300 Class 2

33.6m

Datum R.L. 0.00

DEPTH TO INVERT

GRADIENT & PIPE SIZE

PIPE LENGTH & MH No.

INVERT LEVEL

LID LEVEL

SW LINE 14

Datum R.L. 0.00

1.84% DN 375 Class 2

11.5m

DEPTH TO INVERT

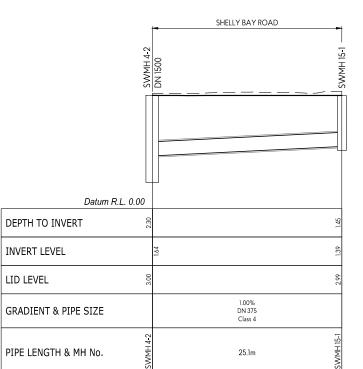
**GRADIENT & PIPE SIZE** 

PIPE LENGTH & MH No.

INVERT LEVEL

LID LEVEL

SW LINE 13



SW LINE 15

#### NOTES:

INDICATES PROPOSED FINISHED GROUND LEVEL

2. — — INDICATES EXISTING GROUND LEVEL (PRE EARTHWORKS)

3. LONG-SECTIONS ARE SHOWN WITH A 5x VERTICAL EXAGGERATION.

- PIPE SIZES, INVERTS & GRADES AND MANHOLE DEPTHS ARE PRELIMINARY DESIGN AND WILL BE CONFIRMED AT DETAILED DESIGN/ENGINEERING APPROVAL STAGE.
- 5. LEVELS ARE IN TERMS OF WELLINGTON VERTICAL DATUM 1953 ORIGIN RM II SO 31470 - RL 3.05.
- ALL WORKS TO COMPLY WITH THE WELLINGTON CITY COUNCIL CODE OF LAND DEVELOPMENT.
- ALL STORMWATER PIPE TO BE RCRRJ CLASS 2 UNLESS SHOWN OTHERWISE.
- 8. ALL MANHOLES TO BE DN 1050 UNLESS SHOWN OTHERWISE.
- CONTRACTOR TO CHECK ALL INVERTS AGAINST PIPE CLASHES BEFORE LAYING. ADJUSTMENT OF ANY INVERT LEVELS IS AT THE CONTRACTORS OWN RISK DUE TO TIGHT TOLERANCES.
- 10. PIPE LENGTH SHOWN IS THE LENGTH OF PIPE BETWEEN CENTRE OF
- 11. HARDFILL BACKFILL ALL TRENCHES BELOW CARRIAGEWAY AND 1m EITHER SIDE OF PIPE CROSSOVERS.

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REV	NOTES	BY	DA
R1	RESOURCE CONSENT ISSUE	PJ	13-09
R2	FOR DEVELOPMENT AGREEMENT	JW	21/0
E1	FOR ENGINEERING APPROVAL	JW	27/0

SHELLY BAY TAIKURU LIMITED SHELLY BAY WELLINGTON

TTI C.

STORMWATER LONG SECTIONS
SHEET 5 OF 5



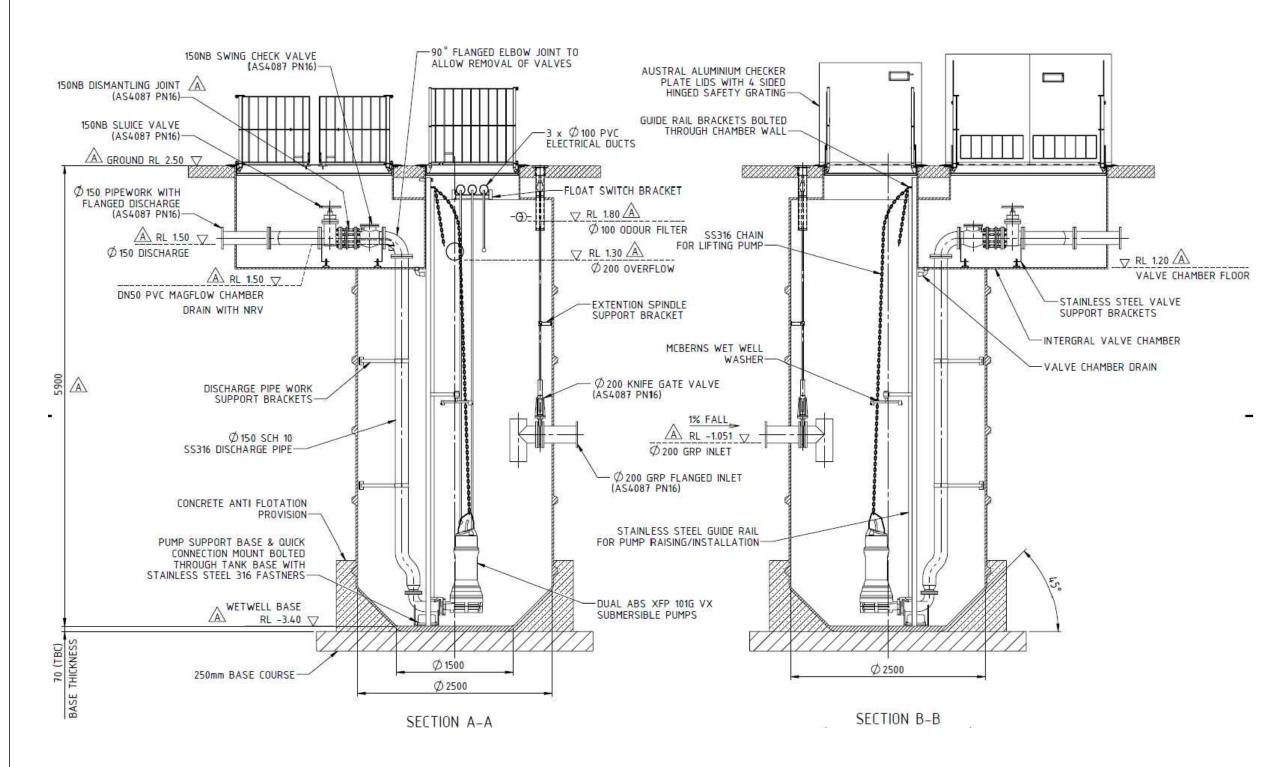
DESIGNED: PJ DRAWN: JW
CHECKED: DM DATE: 7-Sep-2021

SCALE AI: 1:250 Horz, 1:50 Vert SCALE A3: 1:500 Horz, 1:100 Vert

STATUS: ENGINEERING APPROVAL
PROJECT No: DRAWING No:

1098-01 424

revision: **E1** 



- NOT PART OF THE CURRENT CONSENT APPLICATION.
- DETAILS ARE PRELIMINARY AND BASED ON PROPRIETARY SUPPLIER SCHEMATIC. FINAL DESIGN IS SUBJECT TO A DESIGN BUILD TENDER.

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REV	NOTES	BY	DATE
RI	FOR DEVELOPMENT AGREEMENT (INTERNAL)	JW	21/06/202
R2	FOR DEVELOPMENT AGREEMENT	JW	09/07/202
E1	FOR ENGINEERING APPROVAL	JW	27/08/2

SHELLY BAY TAIKURU LIMITED SHELLY BAY WELLINGTON

TTLE:

WASTEWATER PUMP STATION DETAILS SHEET 1 OF 4



 DESIGNED:
 DRAWN:
 JW

 CHECKED:
 DM
 DATE:
 6-Sep-2021

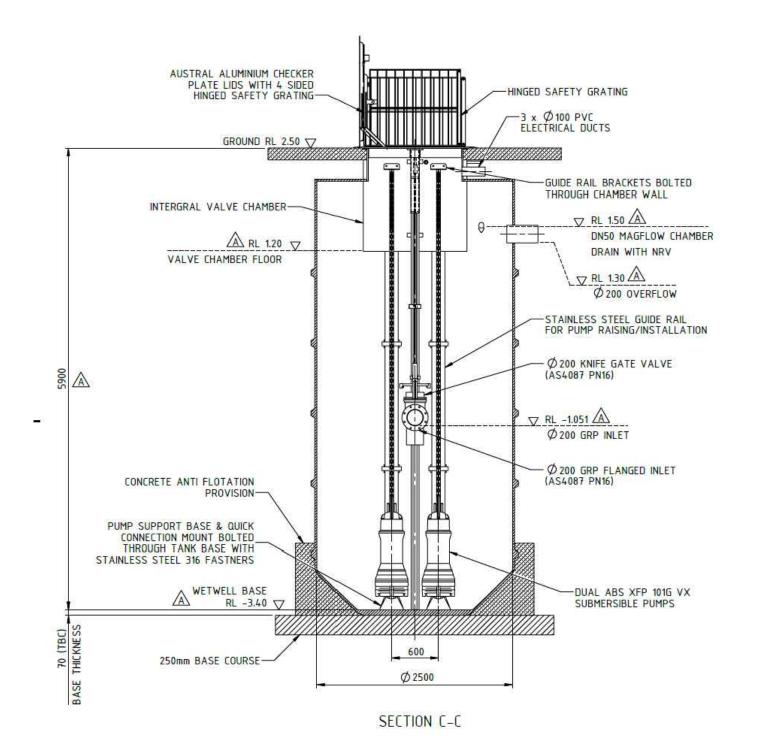
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 NTS
 SCALE A3:
 NTS

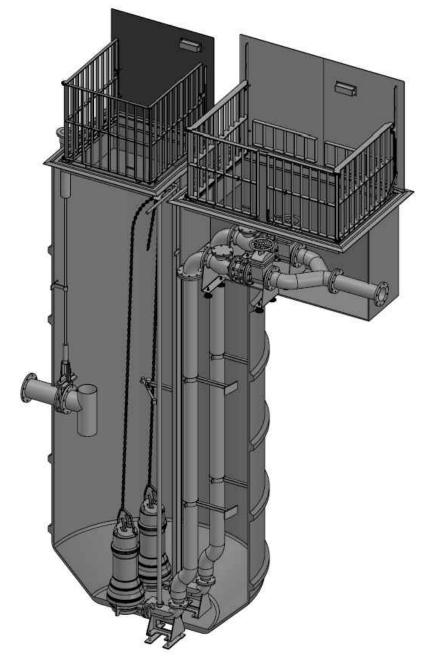
 STATUS:
 ENGINEERING APPROVAL

PROJECT No: DRAWING No: 1098-01 490

E1

REVISION:





- NOT PART OF THE CURRENT CONSENT APPLICATION.
- DETAILS ARE PRELIMINARY AND BASED ON PROPRIETARY SUPPLIER SCHEMATIC. FINAL DESIGN IS SUBJECT TO A DESIGN BUILD TENDER.

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REVI	SIONS:		
REV	NOTES	BY	DATE
R1	FOR DEVELOPMENT AGREEMENT (INTERNAL)	JW	21/06/20
R2	FOR DEVELOPMENT AGREEMENT	JW	09/07/20
El	FOR ENGINEERING APPROVAL	JW	27/08/
	`		

PROJE

SHELLY BAY TAIKURU LIMITED SHELLY BAY WELLINGTON

TITLE:

WASTEWATER PUMP STATION DETAILS SHEET 2 OF 4



W W W . E N V E L O P E - E N G . C O . N

 DESIGNED:
 DRAWN:
 JW

 CHECKED:
 DM
 DATE:
 6-Sep-2021

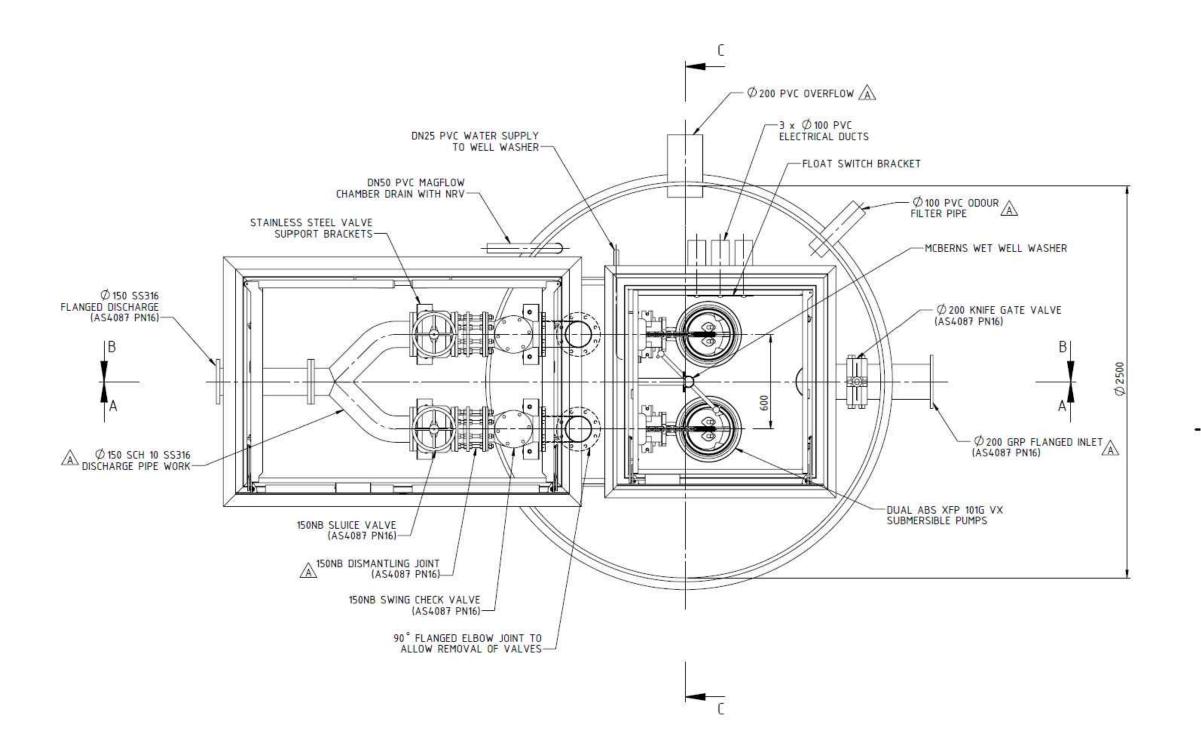
 SCALE AI:
 NTS
 SCALE A3:
 NTS

STATUS: **ENGINEERING APPROVAL**PROJECT No: DRAWING No:

1098-01 491

REVISION:

E1



- NOT PART OF THE CURRENT CONSENT APPLICATION.
- DETAILS ARE PRELIMINARY AND BASED ON PROPRIETARY SUPPLIER SCHEMATIC. FINAL DESIGN IS SUBJECT TO A DESIGN BUILD TENDER.

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KE VI	DIONS:		
REV	NOTES	BY	DATE
R1	FOR DEVELOPMENT AGREEMENT (INTERNAL)	JW	21/06/202
R2	FOR DEVELOPMENT AGREEMENT	JW	09/07/202
E1	FOR ENGINEERING APPROVAL	JW	27/08/2

PROJECT:

SHELLY BAY TAIKURU LIMITED SHELLY BAY WELLINGTON

TIT

WASTEWATER PUMP STATION DETAILS SHEET 3 OF 4



W W W . E N V E L O P E - E N G . C O . N ?

 DESIGNED:
 DRAWN:
 JW

 CHECKED:
 DM
 DATE:
 6-Sep-2021

 SCALE AI:
 NTS
 SCALE A3:
 NTS

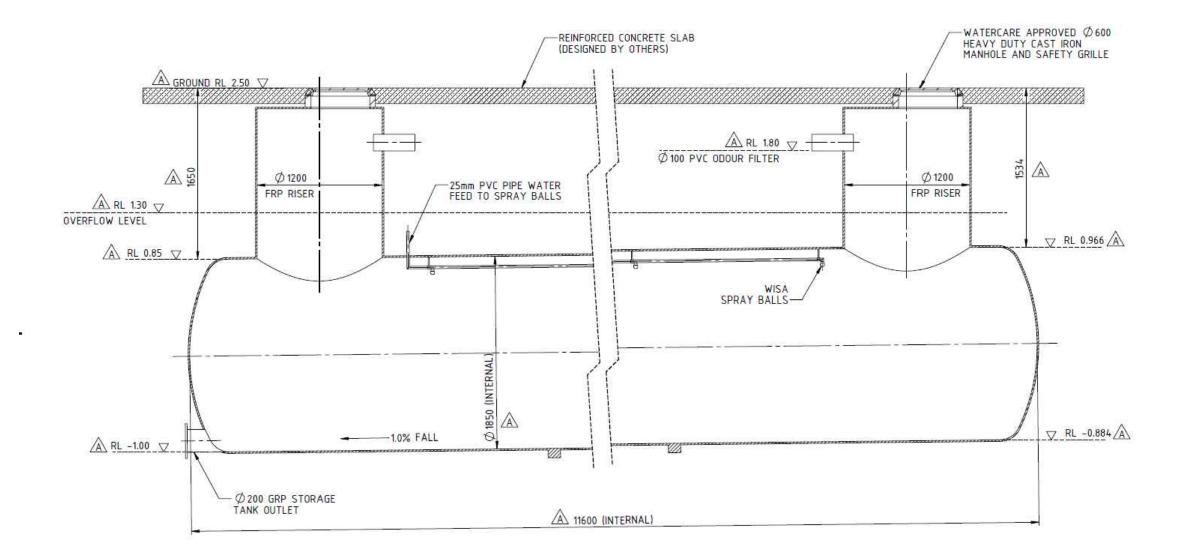
 STATUS:
 ENGINEERING APPROVAL

PROJECT No: DRAWING No:

1098-01 492

REVISION:

E1



- NOT PART OF THE CURRENT CONSENT APPLICATION.
- DETAILS ARE PRELIMINARY AND BASED ON PROPRIETARY SUPPLIER SCHEMATIC. FINAL DESIGN IS SUBJECT TO A DESIGN BUILD TENDER.

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NOTES	BY	DATE
FOR DEVELOPMENT AGREEMENT (INTERNAL)	JW	21/06/2021
FOR DEVELOPMENT AGREEMENT	JW	09/07/202
FOR ENGINEERING APPROVAL	JW	27/08/21
	FOR DEVELOPMENT AGREEMENT	FOR DEVELOPMENT AGREEMENT JW

SHELLY BAY TAIKURU LIMITED SHELLY BAY WELLINGTON

WASTEWATER PUMP STATION DETAILS SHEET 4 OF 4



DESIGNED: -DRAWN: JW CHECKED: DM DATE: 6-Sep-2021 SCALE A1: NTS SCALE A3: NTS STATUS: ENGINEERING APPROVAL

PROJECT No: DRAWING No: 1098-01

REVISION:

E1

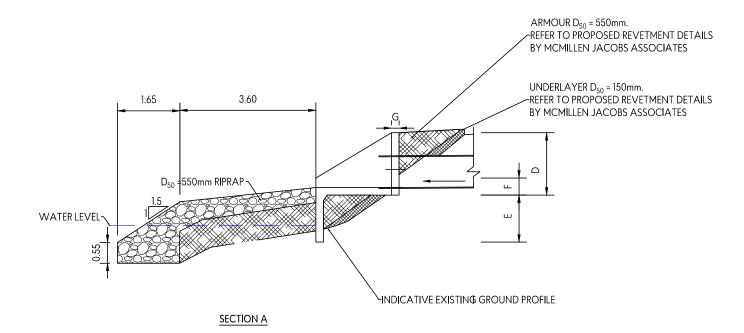
493

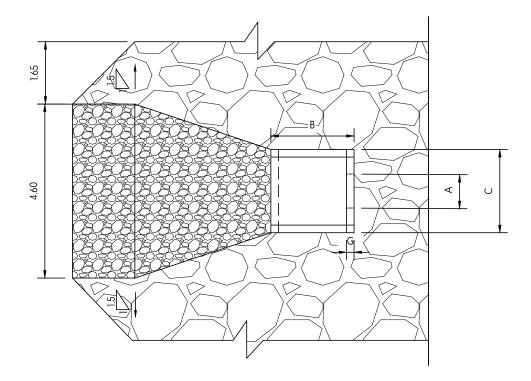
1050 series Concrete Wingwall		
Dimension	Length (mm)	
Α	VARIES*	
В	2200	
С	2200	
D	VARIES**	
E	VARIES***	
Ğ	200	

\*Varies due to pipe sizing

\*\*Varies due to Site Topography

\*\*\*Varies due to sub-surface ground profile.
300mm minimum embedment depth required





SW OUTLET PLAN VIEW

### NOTES:

 DESIGN LAYOUT IS INDICATIVE ONLY AND IS SUBJECT TO FINAL DESIGN BY MCMILLEN JACOBS ASSOCIATES.

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REVIS	SIONS:		
REV	NOTES	BY	DATE
RI	FOR DEVELOPMENT AGREEMENT	JW	23/06/2
E1	FOR ENGINEERING APPROVAL	JW	27/08/2
PRO	JECT:		

SHELLY BAY TAIKURU LIMITED SHELLY BAY WELLINGTON

TITLE

STORMWATER OUTLET DETAILS



 DESIGNED:
 JW
 DRAWN:
 JW

 CHECKED:
 DM
 DATE:
 6-Sep-2021

 SCALE AI:
 1:50
 SCALE A3:
 1:100

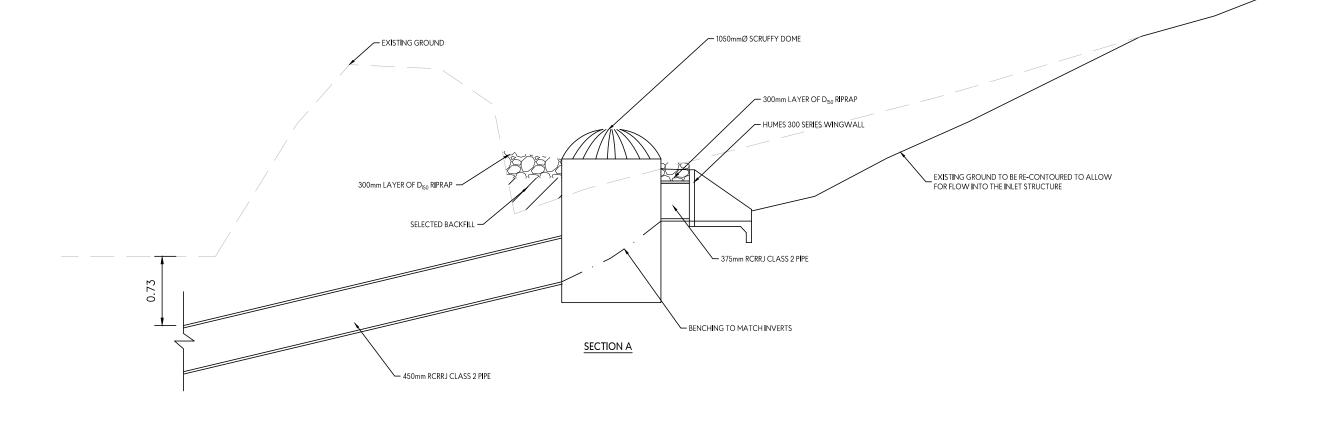
 STATUS:
 ENGINEERING APPROVAL

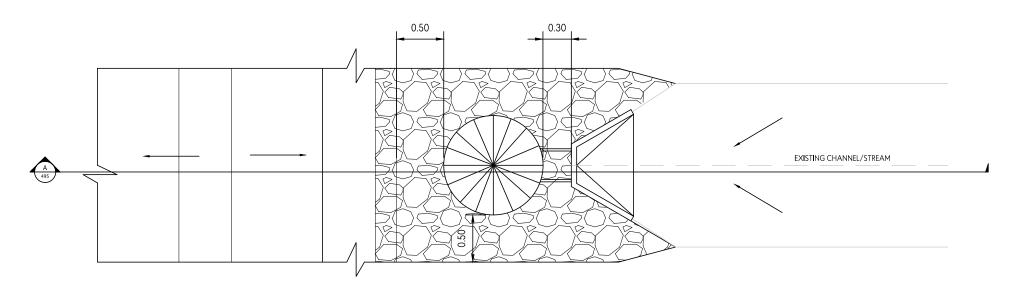
PROJECT No: DRAWING No:

1098-01 494

4

revision:





SW INLET PLAN VIEW

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REV	NOTES	BY
R1	FOR DEVELOPMENT AGREEMENT	JW
E1	FOR ENGINEERING APPROVAL	JW

SHELLY BAY TAIKURU LIMITED SHELLY BAY WELLINGTON

STORMWATER INLET DETAILS



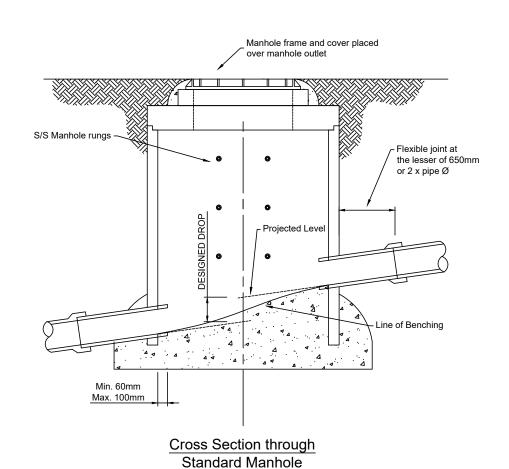
DES**I**GNED: JW DRAWN: JW CHECKED: DM DATE: 31-Aug-2021 SCALE AI: 1:20 SCALE A3: 1:40 STATUS: ENGINEERING APPROVAL

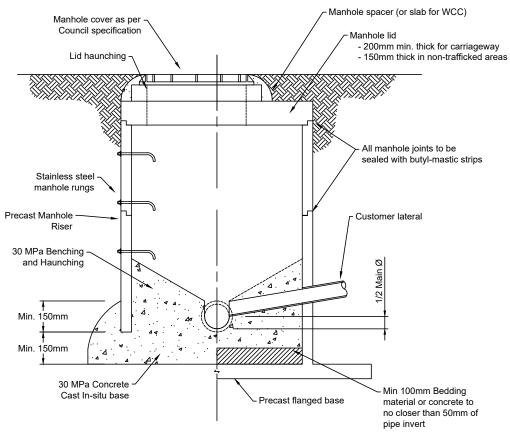
PROJECT No: DRAWING No:

1098-01 495

REVISION:

E1





Typical Manhole Benching and Haunching Details

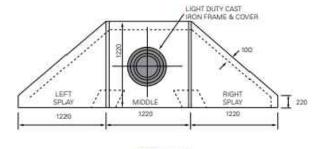
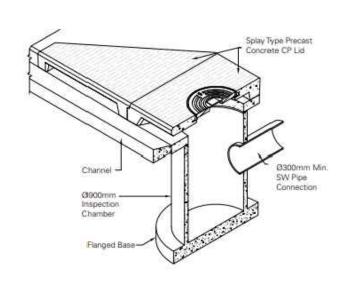
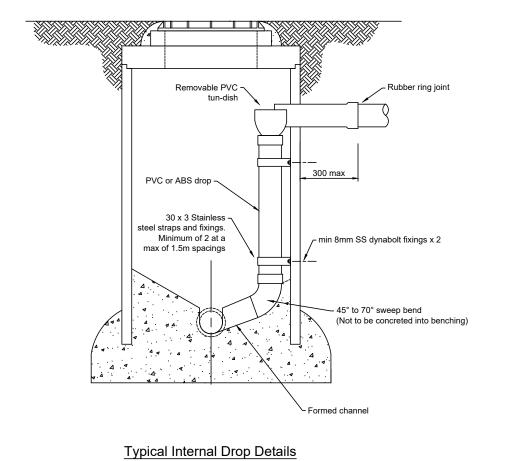
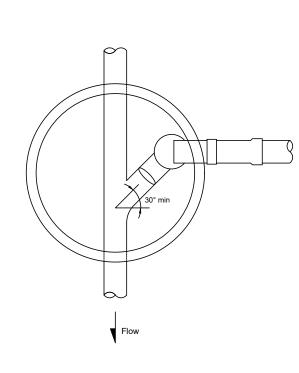


FIG. 2 Plan view (\*)
(\*) Hynds Catchort Lids Design © J McCann 2003







Plan View

DETAILS AS PER WELLINGTON WATER'S
 REGIONAL STANDARDS FOR WATER SERVICES

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E1	FOR ENGINEERING APPROVAL	JW

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TITLE:

MANHOLE AND SPLAY CATCHPIT DETAILS

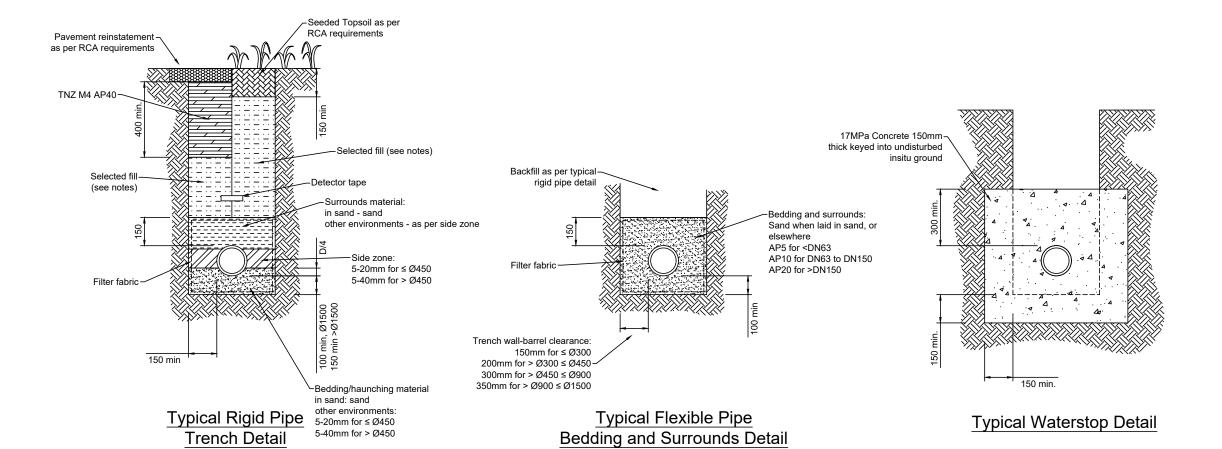


 DESIGNED:
 JW
 DRAWN:
 JW

 CHECKED:
 DM
 DATE:
 31-Aug-2021

 SCALE A1:
 NTS
 SCALE A3:
 NTS

STATUS: ENGINEERING APPROVAL
PROJECT No: DRAWING No:



5-40mm for > Ø450

Trench Detail

### NOTES:

DETAILS AS PER WELLINGTON WATER'S
 REGIONAL STANDARDS FOR WATER SERVICES

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E1	FOR ENGINEERING APPROVAL	JW

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PIPE BEDDING AND TRENCH DETAILS

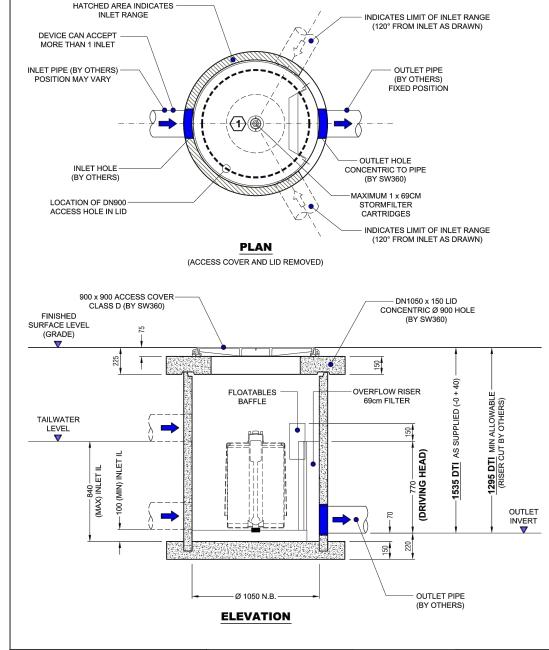


DESIGNED: JW DRAWN: JW DATE: 31-Aug-2021 CHECKED: DM SCALE AT: NTS SCALE A3: NTS STATUS: ENGINEERING APPROVAL

PROJECT No: DRAWING No:

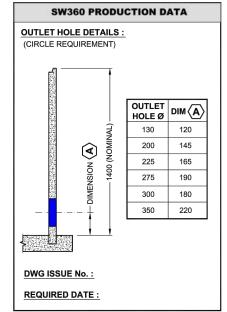
497 1098-01

REVISION: E1



тс	BE COMPL	ETED BY C	USTOMER	CONTRACT	OR	
COMPANY:			P.0	P.O. NUMBER :		
SITE ADDRESS :						
SITE CONTACT & PHONE :						
PREFERRED DELIVERY DATE (TBC SW360):						
STORMFILTER REFERENCE (I	F APPLICABLE)	:				
OUTLET HOLE Ø (CIRCLE) : (TYPICAL PIPE MATERIAL)	Ø 130 (DN100 PVC)	Ø 200 (DN150 PVC)	Ø 225 (DN175 PVC)	Ø 275 (DN225 PVC)	Ø 300 (DN225 RRJ)	Ø 350 (DN300 PVC)
LID LEVEL (RL):		OUTLET PIPE (IL) :		DTI:		
COMPLETED BY :		SIGNED:		DATE :		

TO BE COMPLETED BY SW360					
SW360 PRODUCT CODE :					
MEDIA TYPE (CIRCLE ONE) :	PERLITE	ZPG	OTHER:		
CARTRIDGE QTY (STATE):			PRE-INSTALLATION	ON (Y/N) :	
SP FLOW RATE (CIRCLE ONE):	FULL (Ø 27.6 ID) BLACK/MUSTARD	3 QTR (Ø 24.0 ID) WHITE/OPAL	HALF (Ø 19.7 ID) GREEN	OTHER:	
ACCESS COVER (CIRCLE ONE) :	900 x 900 WEB-FORGE / CLASS D		OTHER:	•	
COMPLETED BY :	SIGNED:		DATE :		



- MANHOLE UNIT FITTED WITH SWIFTLIFT ANCHOR POINTS (QTY 2). DO NOT EXCEED 60 DEGREE LIFT ANGLE. CONCRETE LID FITTED WITH SWIFTLIFT ANCHOR POINTS (QTY 4).

  UNIT SUPPLIED WITH OUTLET CORE DRILLED. INLET/S CORE DRILLED ON SITE BY OTHERS WITHIN RANGE SHOWN ON DRAWING. SEALING / GROUTING OF MANHOLE COMPONENTS AND PIPES BY
- CONTRACTOR. ENSURING LOCAL CODES AND REGULATIONS ARE COMPLIED WITH.
  ANY RISERS REQUIRED TO INCREASE THE DEPTH TO INVERT (DTI) FROM THAT AS DRAWN TO BE SUPPLIED BY THE CONTRACTOR.
- FOR A DTI EXCEEDING 5m PLEASE CONTACT **0800STORMWATER**. CONTRACTOR TO TAKE APPROPRIATE MEASURES TO PROTECT CARTRIDES FROM CONSTRUCTION RELATED EROSION RUINOFF. BACKFILL, BEDDING AND BUOYANCY DESIGN BY ENGINEER OF

- RECORD

  8. QTY OF CARTRIDGES BY ENGINEER OF RECORD.

  9. CONCRETE MANHOLE RISERS ARE DESIGNED AND MANUFACTURED IN ACCORDANCE WITH AS/NZS 4058: 2007

  10. CONCRETE MANHOLE BASES ARE DESIGNED AND MANUFACTURED IN ACCORDANCE WITH NZS 3101: 2006 & NZS 3109: 1997

  11. CONCRETE LID DESIGNED AND MANUFACTURED TO HN-HO-72

  12. FOR REQUIREMENTS OUTSIDE OF DRAWING SPECIFICATIONS PLEASE CONTACT 0800STORMWATER.
- CONTACT <u>0800STORMWATER</u>.

## **APPROX WEIGHTS**

MANHOLE SECTION INCLUDING CARTRIDGES: 1600 Kg (AS DELIVERED, BASED ON QTY 1 ZPG CARTS) LID WEIGHT: 500 Kg

Stormwater360° **BETWEEN SKY AND SEA** 

	0800 STORMWATER	ŀ
$\geq$	sales@stormwater360.co.nz	
	www.stormwater360.co.nz	

CONDITION OF USE © STORMWATER360 2019 Any unauthorised reproduction of this drawing in part or in full is prohibited

STORMFILTER® CARTRIDGE FILTRATION SYSTEM 69cm CART / DN1050 x 1500 MH - TRAFFICABLE GENERAL ARRANGEMENT SCALE: NTS DRG No: SF-MH-69-1050-T-20 REV: 1 DATE: 01.03.19 CKD:

REV REVISION DETAIL DATE JOB NO: 01.03.1 APPROVED ROJECT MARK-UPS 09.04.19 R.P. 01.03.19 G.S. G.Y. 01.03.19

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REV	NOTES	BY	
RI	FOR DEVELOPMENT AGREEMENT	JW	2
E1	FOR ENGINEERING APPROVAL	JW	2
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			П

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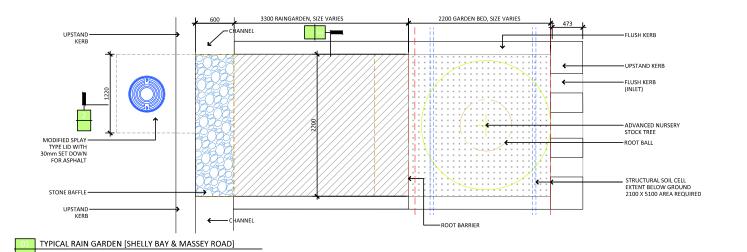
STORMWATER 360 STORMFILTER DETAILS

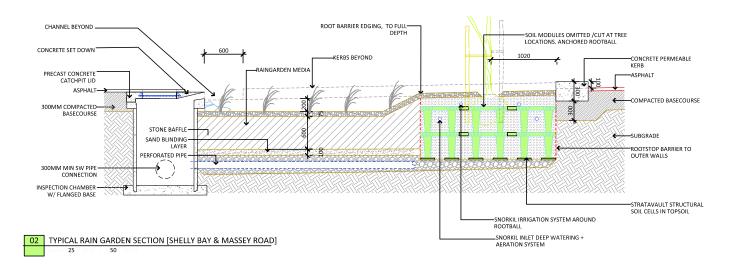


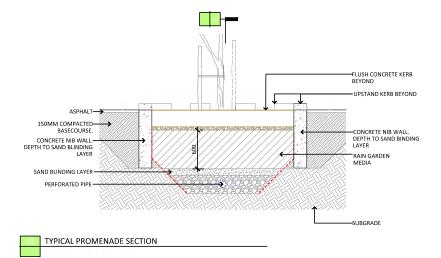
DESIGNED: DRAWN: JW CHECKED: DM DATE: 31-Aug-2021 SCALE A3: NTS SCALE AT: NTS

STATUS: ENGINEERING APPROVAL PROJECT No: DRAWING No:

1098-01 498 REVISION: E1







- REFER TO WAAL/LANDSCAPE ARCHITECTS FOR LANDSCAPE AND RAINGARDEN DETAILS. SUBJECT TO APPROVAL BY WELLINGTON WATER/REGIONAL COUNCIL.
- RAINGARDEN MEDIA AS PER WELLINGTON
  WATER WSD SPECIFICATIONS AND
  LANDSCAPE ARCHITECTS ADDITIONAL DETAILS.
- 3. MODIFIED LID WITH 30mm SET DOWN FOR ASPHALT (NO STEEL FORMED EDGE) TO BE USED.
- 4. 150mm PRECAST/CAST IN PLACE BOX REINFORCED WITH HD12 AT 300mm c/s EACH WAY TO BE USED WHERE REQUIRED.

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REV	NOTES	BY	DATE
E1	FOR ENGINEERING APPROVAL	JW	03/09/

SHELLY BAY TAIKURU LIMITED SHELLY BAY WELLINGTON

TITLE

RAINGARDEN DETAILS



 DESIGNED:
 JW
 DRAWN:
 JW

 CHECKED:
 DM
 DATE:
 6-Sep-2021

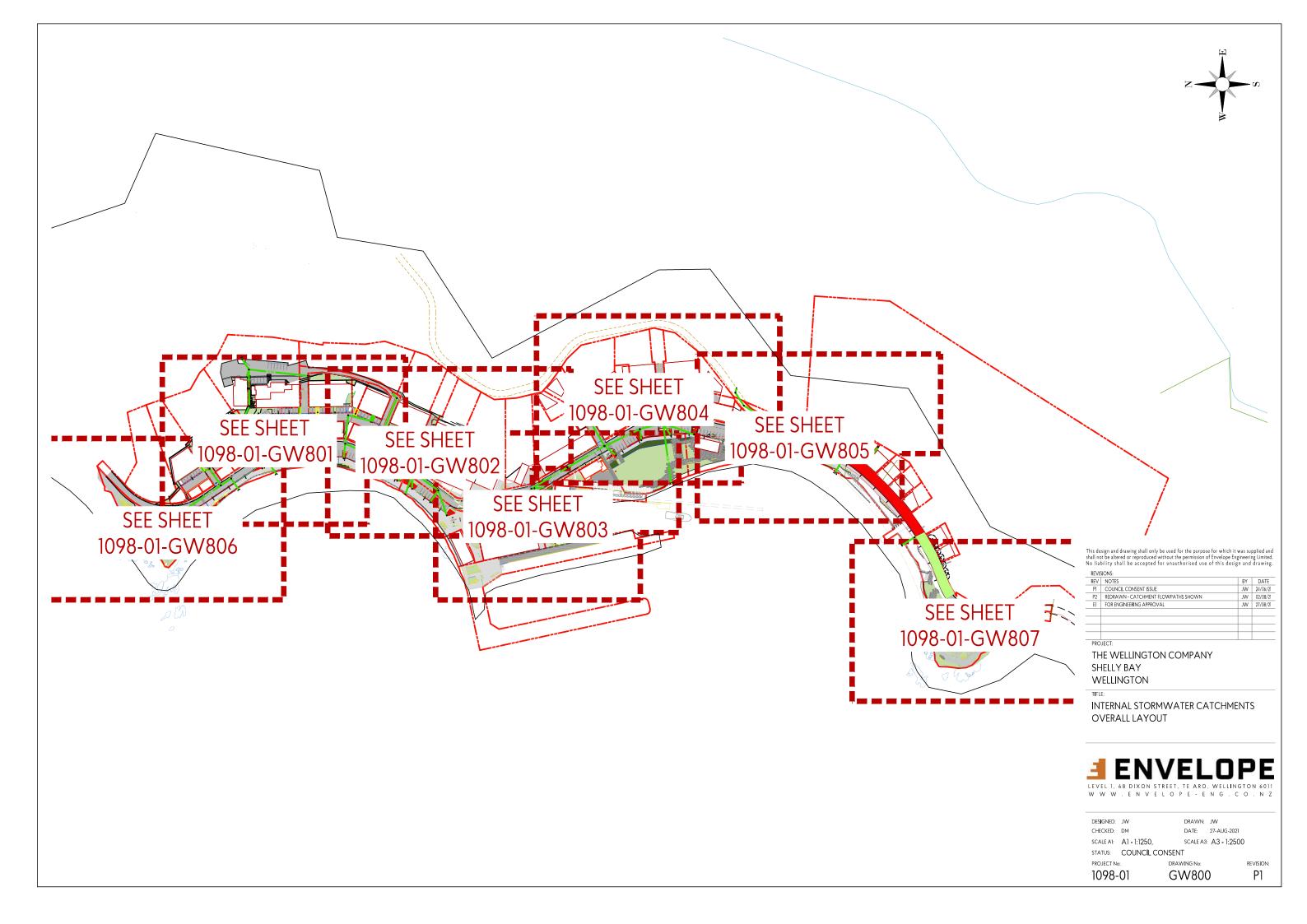
 SCALE AI:
 NTS
 SCALE A3:
 NTS

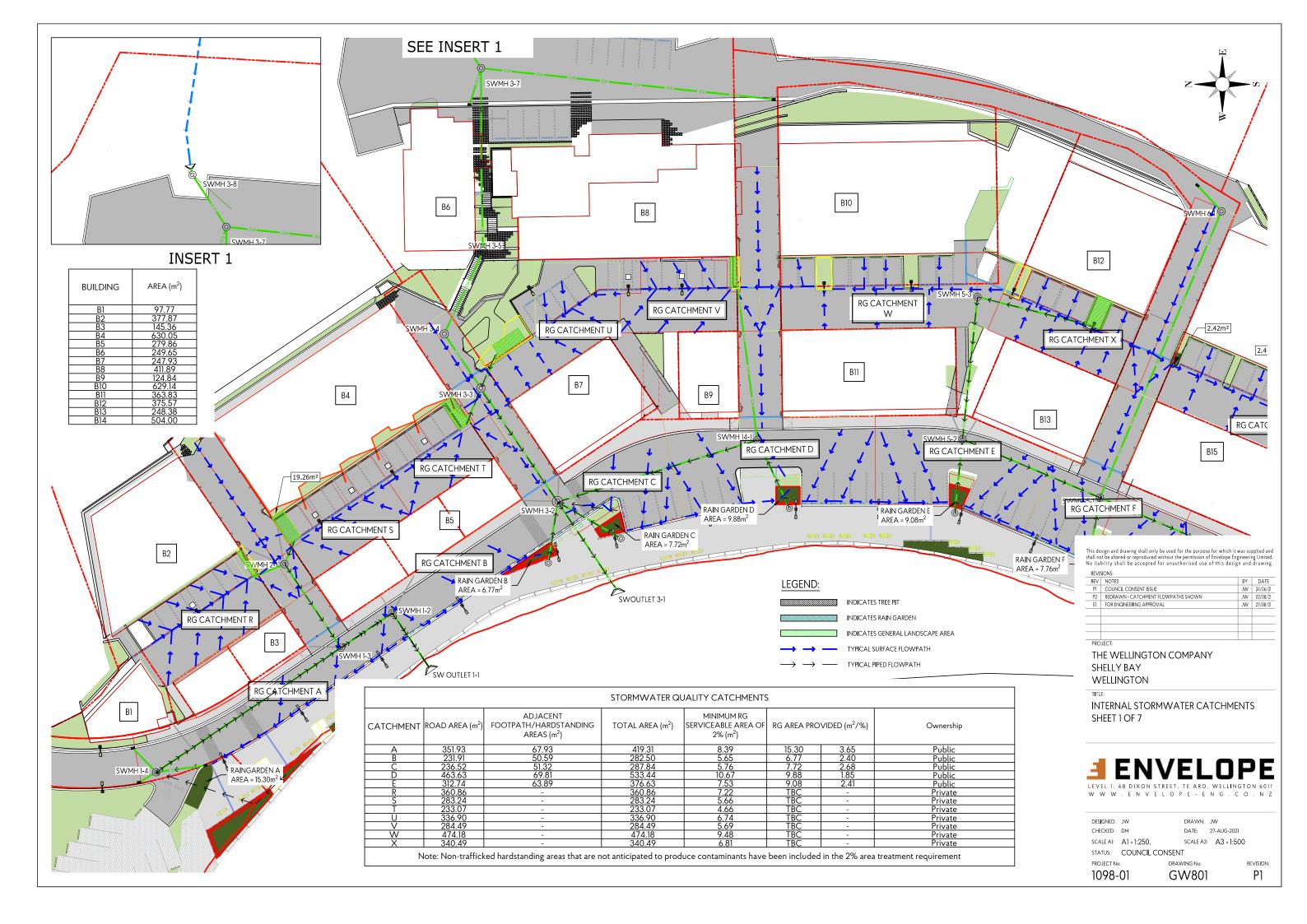
 STATUS:
 ENGINEERING APPROVAL
 TATUS:
 TATUS:

PROJECT No: DRAWING No:

1098-01 499

revision:





BUILDING AREA (m²)	STORMWATER QUALITY CATCHMENTS	<b>a</b>
B15 391.57	CATCHMENT ROAD AREA (m²) FOOTPATH/HARDSTANDING AREAS (m²) ADJACENT FOOTPATH/HARDSTANDING AREAS (m²) TOTAL AREA (m²) AREAS (m²) RAINGARDEN AREA PROVIDED (m²) OWNERSHIP	
B16     378.00       B17     504.93       B19     276.24       B20     1885.87		-01
B20 1885.87 B21 99.00	F         389.63         32.40         422.03         8.44         7.76         1.84         Public           G         224.92         32.33         257.25         5.15         13.76         5.35         Public           H         254.45         31.23         285.68         5.71         13.79         4.83         Public           I         172.32         97.54         269.86         5.40         7.25         2.69         Public           J         301.29         285.82         587.11         11.74         13.86         2.36         Public           W         474.18         -         474.18         9.48         TBC         -         Private	<b>&gt;</b>
	X     340.49     -     340.49     6.81     TBC     -     Private       Y     516.09     -     516.09     10.32     TBC     -     Private       Z     387.52     -     387.52     7.75     TBC     -     Private	
	Note: Non-trafficked hardstanding areas that are not anticipated to produce contaminants have been included in the 2% area treatment requirement	
B12		
SATCHMENT SWMH 5-3		
RG CATCHMENT X	B14	
	2.41m <sup>2</sup>	
	7.05m²	
B13	CATCHMENT Y RIA	
RG CATCHMENT E	B16 B16	
	10.20m²	LEGEND:
RAIN GARDEN E RG CATCHMENT F	RG CATCHMENT Z	INDICATES TREE PIT  INDICATES RAIN GARDEN
AREA = 9.08m <sup>2</sup>		→ → TYPICAL SURFACE FLOWPATH
RAIN GARDEN F AREA = 7.76m <sup>2</sup>	NT G	TYPICAL PIPED FLOWPATH
RAIN GARDEN G AREA = 13.76m <sup>2</sup>	B17 B19	This design and drawing shall only be used for the purpose for which it was supplied and shall not be altered or reproduced without the permission of Envelope Engineering Limited. No liability shall be accepted for unauthorised use of this design and drawing. REVISIONS:
	RG CATCHMENT H	REV         NOTES         BY         DATE           P1         COUNCIL CONSENT ISSUE         JW         24/06/21           P2         REDRAWN - CATCHMENT FLOWPATHS SHOWN         JW         20/208/21           E1         FOR ENGINEERING APPROVAL         JW         27/06/22
	RAIN GARDEN H AREA = 13.79m <sup>2</sup>	
	RG CATCHMENT I	PROJECT: THE WELLINGTON COMPANY
SW OUTLE	RAIN GARDEN I AREA = 7.25m <sup>2</sup>	SHELLY BAY WELLINGTON TITLE:
	RG CATCHMENT J	INTERNAL STORMWATER CATCHMENTS SHEET 2 OF 7
	RAIN GARDEN J	ENVELOPE
	AREA = 13.86m <sup>2</sup>	LEVEL 1, 68 DIXON STREET, TE ARO, WELLINGTON 6011 W W W . E N V E L O P E - E N G . C O . N Z
		DESIGNED: JW DRAWN: JW CHECKED: DM DATE: 27-AUG-2021  SCALE AI: A1 - 1:250, SCALE A3: A3 - 1:500
		STATUS: COUNCIL CONSENT PROJECT No: DRAWING No: REVISION:
		1098-01 GW802 P1

